



# Dudley Avenue Flood Resilience Project



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Prepared for the City of Newport by Fuss & O'Neill





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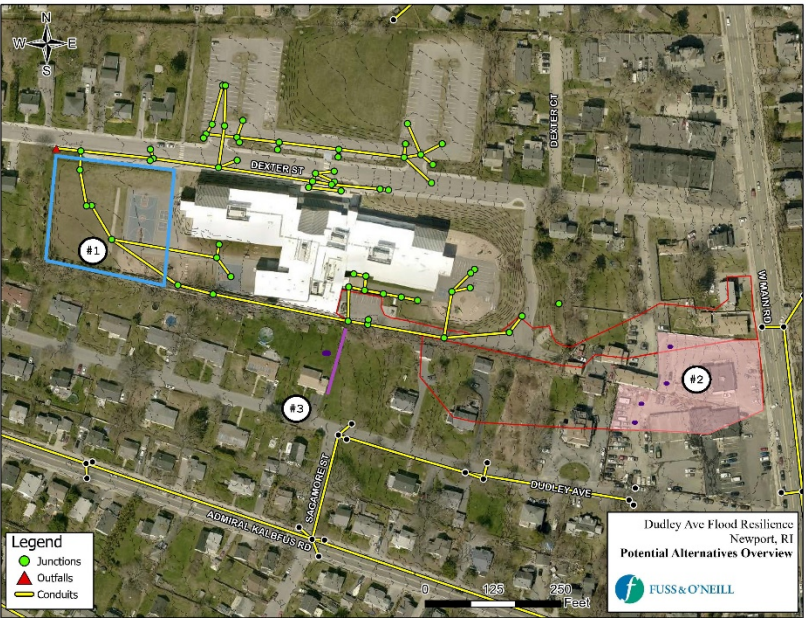


# Executive Summary & Project Background

The residential neighborhood immediately downstream of the Pell School on Dudley Avenue has experienced substantial flooding over the years during storm events. Some neighbors have linked this flooding to past school expansions, and current plans for improvements at the school have heightened some concerns about potentially increasing flood impacts. This report summarizes an analysis looking to better understand the causes of the flooding experienced by the school’s neighbors along Dudley Avenue and simultaneously considers potential alternatives that could reduce the scale and frequency of these events.

To evaluate flooding and potential solutions, hydrologic and hydraulic models were developed for existing and proposed conditions. Four storm types were evaluated for this system: the 1-inch, 2-inch, 3.4-inch, and 5-inch rainfall events (reference section 2, Storms and Flooding for a more detailed explanation). Photographs gathered from residents who have experienced frequent flooding on private properties were used as reference points during the model development both to help calibrate the model as well as observe results from actual storm events.

The analysis of the existing conditions model showed surface flooding in locations that correspond with community feedback about observed flooding locations along Dudley Avenue. In the 1- and 2-inch storms, surface flooding was observed predominantly in the backyards of several private residences along Dudley Avenue and eventually the eastern portion of Dudley Avenue near the intersection with Sagamore Street. The analysis suggests that the stormwater runoff causing this flooding does not originate from the school property, but rather originates from the



**Figure ES1. Proposed Alternative Locations**

eastern properties along West Main Street where there is no stormwater infrastructure. In the 3.4- and 5-inch storm event, surface flooding was observed at the same locations as mentioned above and additionally on Dexter Street north and west of the school and on the eastern school playground.

The analysis does reveal a connection forming between the flooding on the southern edge of the school property and the Dudley Avenue backyards. However this connection occurs only after the flood waters in the backyards have already reached peak depth and extent. This indicates that the cause of the connection stems from the water accumulating first in the Dudley Avenue backyards and is not occurring because of water flowing south from the school property.

Several alternative structural improvements to the existing drainage system were assessed to potentially reduce flooding in the project area. Since the developed areas of the school do not appear to be the primary driver of flooding, the three alternatives were developed with an emphasis placed on addressing the runoff draining directly to the backyards of the private residences.

## **These alternatives include:**

**Alternative 1, Proposed School Stormwater Infrastructure:** The City of Newport is proposing an eight-classroom expansion project at Claiborne Pell Elementary School. As part of this project, the City is proposing regrading and partial paving of the western grass field, as well as installation of new stormwater infrastructure to capture water accumulating in this area. Alternative 1 assumes that the stormwater improvements proposed as part of the classroom expansion are complete and successfully capture all stormwater runoff originating from the field. The results indicate that the removal of the western grass field produced no significant reduction in maximum flood inundation extent, depth, or duration within the residential backyards along Dudley Avenue.

**Alternative 2, Stormwater Infrastructure Along West Main Road Properties:** The focus of Alternative 2 includes the installation of stormwater infrastructure (catch basins) along the western boundary of the impervious commercial area along West Main Road to capture runoff before it reaches the backyards along Dudley Avenue. The model results indicate that the proposed stormwater infrastructure improvements along West Main Road would help to reduce stormwater flooding in the backyard areas under the more frequent, lower depth storm events. It should be noted that these improvements are not in the City of Newport and coordination would be required with Middletown, RI for any further consideration.

**Alternative 3, Stormwater Infrastructure on Private Property Along Dudley Avenue:** The focus of Alternative 3 includes the excavation of a small swale to prevent the propagation of surface flooding from traveling west across the low-lying areas within the Dudley Avenue backyards. The swale outlets into a proposed subsurface infiltration chamber that retains approximately 2,200 ft<sup>3</sup> of stormwater. The model results indicate that the proposed stormwater improvements within the Dudley Avenue backyards would minimally reduce maximum stormwater flooding for storm events greater than the 1-inch storm.

In addition to the alternatives examined in this study, there are opportunities for private homeowners along Dudley Avenue to implement small-scale green infrastructure practices, like swales or raingardens, on their own properties. These small-scale practices have the potential to produce localized reductions to flood depths and extents. However, these practices would produce more significant beneficial cumulative effects when implemented together with the proposed alternatives mentioned above.

The opinion of cost to implement Alternative 1 is not included in this report as the improvements included in Alternative 1 are part of the City's proposed eight-class expansion project (separate from this project) that is still under development. An Order of Magnitude level opinion of cost has been prepared for the proposed improvements as part of Alternatives 2 and Alternative 3 and range from \$240,000 to \$50,000 respectively.



# 1 What Is the Purpose of this Project?

The residential neighborhood immediately downstream of the Pell Elementary School on Dudley Avenue has experienced flooding over the years during significant storm events. Neighbors have raised concerns that this flooding could potentially be related to past school expansions, and recent plans for improvements at the school have heightened neighbors' concerns about increasing flood impacts. The purpose of this study is to better understand the causes of the flooding experienced by the Pell School downstream neighbors on Dudley Avenue and explore potential alternatives that could reduce the scale and frequency of this flooding. (Figure 1 & 2)



Figure 1. Dudley Avenue Project Area



Figure 2. Flooding in Rear of Private Property



## 2 Existing Conditions

The Dudley Avenue area drainage contains primarily medium-density residential and some commercial and school land uses. The average impervious percent in this area is currently approximately 59%. The project's drainage area extends north and east of Dudley Avenue towards the vicinity of the Dexter Street and West Main Road intersection. This drainage area generally slopes from the northeast towards the southwest. Runoff from the drainage area is collected in a closed drainage system on the school property that conveys the stormwater first to infrastructure on Dexter Street, then towards Hillside Ave and away from the system ([Figure 3](#)).

The Dudley Avenue neighborhood is in a low-lying area at the bottom of the watershed, receiving all uncaptured drainage from these upgradient areas. Several factors, including high levels of imperviousness and poor soils, exacerbate flooding in this area.

Runoff from the 17-acre drainage area travels overland before it is collected by a closed storm drainage system buried under Dexter Street and the Pell Elementary School property in this neighborhood. This system eventually discharges to municipal storm infrastructure located on Hillside Ave which convey flood flows away from the area of interest.



Figure 3. Dudley Avenue Area Drainage Area

## Existing Drainage System

The existing drainage within the Pell Elementary School property generally consists of 12- to 18-inch drainage pipes that connect to 24-inch pipes along Dexter Street which convey water towards Hillside Avenue and away from the system. Due to the direction of runoff in the drainage area, the existing stormwater infrastructure on the of Dudley Avenue is downgradient was determined to not influence the flooding experienced in the private residences north of Dudley Avenue and thus was not included in this flooding analysis.

## Topography

The topography of the study area generally slopes from the intersection of Dexter Street and West Main Street at the northeastern boundary of the drainage area, with water flowing in a north to south direction. One characteristic of note in the project area is that the topography between Pell Elementary School and Dudley Avenue is a natural low area that extends to Hillside Avenue (Figure 4). This low area may reflect a historic watercourse which would also be consistent with the poor soils in this area.



Figure 4. Elevation Contour Map

This low area presents a challenge in this watershed. Runoff will naturally want to accumulate in the low area thereby exacerbating the volume of water that needs to be managed in these areas. Additionally, there is little storm infrastructure capturing runoff originating in the eastern portion of the drainage area before it travels downgradient to this low point south of the Pell school property.

## Soils

The soils in the watershed are mostly classified as Newport-Urban Land Complex and are characterized as Hydrologic Soil Group (HSG) C, which has slow infiltration capacity. In general, City of Newport soils are characterized primarily as HSG C. In comparison, HSG A and B soils have high to moderate infiltration capacity. The saturated hydraulic conductivity within the watershed could range from between 0.0 to 0.2 inches per hour; however in-situ testing is required to confirm these characterizations. As a result, there is a significant potential for runoff even from unpaved surfaces in this watershed.

## Storms and Flooding

There are generally two types of storm events that are typically observed for the study area.

- **Large and sustained inland floods** - These are major floods with sustained precipitation. Recent inland floods of this type were the June 7-10, 2013 storm (5.1 inches), which was close in total rainfall to a 10-year frequency storm (5.0 inches) and the October 29-30, 2017 storm event (3.0 inches), which was close in total rainfall to a 2-year frequency storm (3.4 inches). A 10-year frequency storm is a typical standard for the design of “new” drainage systems.

**Flash Floods** – Flash flooding in the study area is primarily associated with summer thunderstorm systems that are characterized by large, usually isolated, thunderstorm patterns with a high intensity rainfall over a short duration. An example storm occurred on July 18, 2020, which had recorded rainfall of 1.53-inches over 5 hours.

### What is a “2-Year Storm”?

A two-year frequency storm has a total amount of precipitation that has a 50% probability of being equaled or exceeded in any given year. While this storm could happen more than once in a given year, over a long period of record it would be equaled or exceeded on an average of once every two years.

For Newport, Rhode Island, a total amount of rainfall equaling 3.4-inches over a 24-hour period constitutes the 2-year rainfall amount. The typical rainfall pattern is the majority of the rain would fall within approximately 4-to-6-hour period in the middle of the 24-hour duration storm. The modeled storm events are discussed more in Section 3.

## Observed Localized Flooding

A public meeting was held on October 7th, 2021. The purpose of meeting was to provide information to members of the public about this study and receive public observation and input to the study by requesting testimonials and photographs of past flooding events both during the meeting and following the meeting via email.

Photographs and narrative accounts of flooding were received from residents. The general area of concern was identified as the backyards of the private residential properties between 26 and 50 Dudley Avenue. Flood waters accumulating in this area has the potential to spill over onto Dudley Avenue near its intersection with Sagamore Street. The information gathered during these meetings was used in calibrating the SWMM model (See [Section 3](#)) which was then used to develop alternatives to address the flooding.



# 3 Dudley Avenue Area Drainage Model

A Storm Water Management Model (SWMM) Version 5.1.014 model was developed using PCSWMM version 7.3, for the 17-acre Dudley Avenue area drainage area, which include modeling rainfall and runoff for the entire catchment area and detailed hydraulics of the drainage system in project area.

## Data Collection

A two-dimensional (2-D) surface was created within PCSWMM to model surface flooding caused by surface runoff and infrastructure surcharging within the stormwater system. Elevation data for the 2-D surface was obtained from a 1-meter digital elevation model (DEM) originating from a composite of the Spring 2011 Rhode Island Statewide USGS LiDAR<sup>2</sup> and contours digitized from a 2021 Perimeter and Topography survey plan<sup>3</sup>. One-dimensional (1-D) elements, including catch basins, manholes, and pipes, were provided as part of the City's ArcGIS storm drainage data layer. Additional 1-D elements not originally included in the City's the data layer were later added from the survey plan and supplemental drainage analysis<sup>4</sup>. Anecdotal and photographic evidence gathered from residents regarding frequently flooded regions were used as reference points during model development.

## Modeling Approach

One-dimensional (1-D) elements, including catch basins, manholes, and pipes, were provided as part of the City of Newport's ArcGIS storm drainage data layer. Additional 1-D elements not originally included in the City's data layer were added from the survey plan and supplemental drainage analysis<sup>5</sup>. Photographic evidence gathered from residents in frequently flooded areas were used as reference points during model development both to help calibrate the model as well as observe results from an actual storm event.

Subcatchments were delineated and the average slope determined using topographic information obtained from a composite of the Spring 2011 Rhode Island Statewide USGS LiDAR and contours digitized from a 2021 Perimeter and Topography survey plan mentioned above. Subcatchments are named based on their respective outlet ([Figure 5](#)). The percent impervious value is based on the Rhode Island Department of Environmental Management impervious surface raster data layer, with impervious percentages ranging from 2 to 100%. The subcatchment widths were determined based on typical flow paths to reach the storm drain system.

The system was modeled using a combined one and two-dimensional model in which the closed drainage system connects to the land surface to model the depth and extent of flooding from surcharging catch basins and overland runoff. The hydrologic properties of the subcatchments within the modeled drainage basin were determined from available topographic, land use, soils, and hydrography data. Rainfall infiltration rates were calculated using the Modified Green-Ampt Method. Soil data from the National

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<sup>2</sup> 2011 U.S. Geological Survey Topographic LiDAR: LiDAR for the North East from 2010-06-15 to 2010-08-15. NOAA National Centers for Environmental Information, <https://www.fisheries.noaa.gov/inport/item/49844>.

<sup>3</sup> "Perimeter and Topography Survey for Claiborne Pell Elementary School Situated on 35 Dexter Street Newport, Rhode Island Prepared for Studio Jaed", Dated March 2021, By Garofalo & Associates, Inc. Civil & Structural Engineers/Surveyors, Providence, RI.

<sup>4</sup> Garcia Galuska Desousa, Consulting Engineers Inc., 2012. Revised Drainage Analysis for the New Claiborne Pell Elementary School

<sup>5</sup> Garcia Galuska Desousa, Consulting Engineers Inc., 2012. Revised Drainage Analysis for the New Claiborne Pell Elementary School

Cooperative Soil Survey - Web Soil Survey<sup>6</sup> was used to assign infiltration parameters to the soils throughout the watershed. Land use data was obtained from the Rhode Island Geographic Information System (RIGIS<sup>7</sup>). Modeling input is included in Attachment A.



Figure 5. SWMM Model Schematic

<sup>6</sup> Natural Resources Conservation Service. <https://websoilsurvey.sc.egov.usda.gov/App/HomePage.htm>.

<sup>7</sup> RIGIS, 2021. Rhode Island Geographic Information System (RIGIS) Data Distribution System. URL: <http://www.rigis.org>. Environmental Data Center, University of Rhode Island, Kingston, Rhode Island (last date accessed: November 2021).

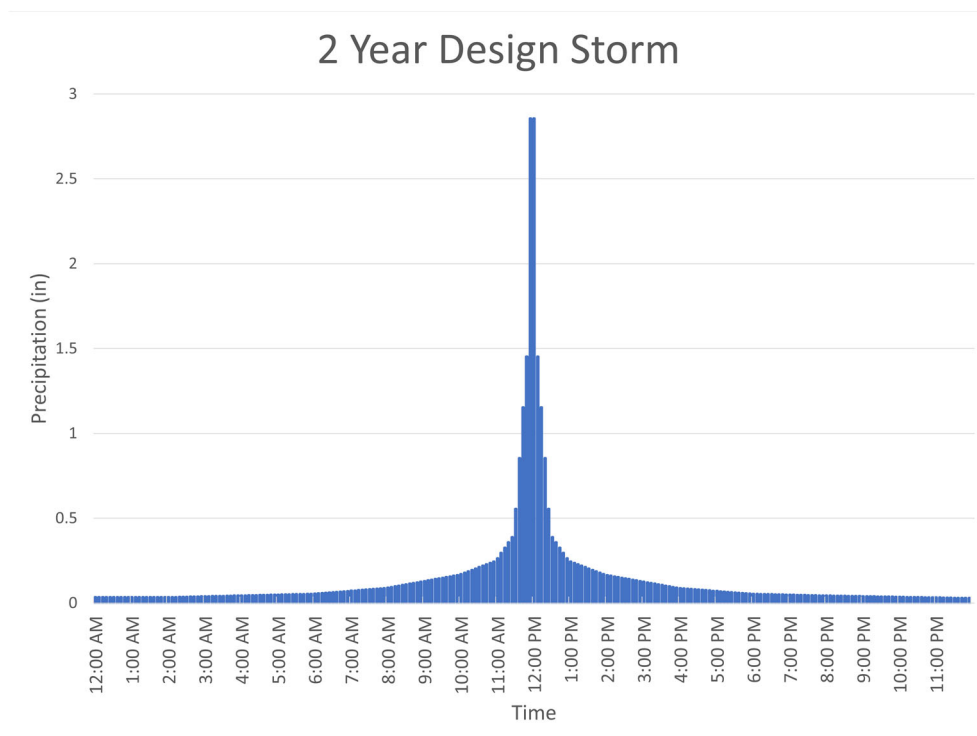
## Modeled Storm Events

Analyses for the existing conditions and the proposed alternatives were run for the 1-inch, 2-inch, 3.4-inch, and 5-inch rainfall events. The rainfall depths of 3.4-inch and 5-inch correlate to the 2-year and 10-year storms and were obtained from NOAA *Atlas 14 Point Precipitation Data*<sup>8</sup>. The distribution of rainfall over time (referred to as hyetographs) were based on the Natural Resources Conservation Service Type III Synthetic 24-hour Rainfall Distribution<sup>9</sup>. The design precipitation depths are summarized in *Table 1*.

**Table 1. Newport Storm Events**

Storm Event	Total Rainfall Depth (in)	Duration (hr)
1-inch	1.00	24
2-inch	2.00	24
2-year	3.40	24
10-year	5.03	24

Recurrence interval (also called return period) describes the probability that a specific rainfall amount over a 24-hour period will be equaled or exceeded. For example, there is a 1 in 2 chance that in any given year 3.4 inches of rain will fall on this watershed in 24 hours (NOAA 2014). The frequency this occurs is referred to as a 2-year return period or 2-year storm.



**Figure 6. 2 Year Storm Event Hyetographs Model Input**

<sup>8</sup> National Weather Service. (2015). *NOAA Atlas 14 Precipitation-Frequency Atlas of the United States, Volume 10 Version 2.0: Northeastern States (Connecticut, Maine, Massachusetts, New Hampshire, New York, Rhode Island, Vermont)*. Department of Commerce, National Oceanic & Atmospheric Administration. Silver Springs, Maryland.

<sup>9</sup> Natural Resources Conservation Service. (June 1986). *Technical Release 55: Urban Hydrology for Small Watersheds*. U.S. Department of Agriculture, Washington D.C.



## Model Calibration

The model calibration was checked using photographs taken by private residential property owners during recent historical storms. [Figure 7](#) is a photograph of the backyard of 46 Dudley showing flooding conditions during the April 14th, 2020 storm event. This event produced a recorded 1.01 inches of rainfall which is comparable to the modeled 1-inch storm event. The simulation model results are 8 inches of maximum flooding depth at this location, which is comparable to the estimated depth of water captured in this photograph. It is possible that the photograph was not taken at exactly the peak flood depth of the storm, so there is room for minor discrepancies between model results and the flood depth in the photograph.

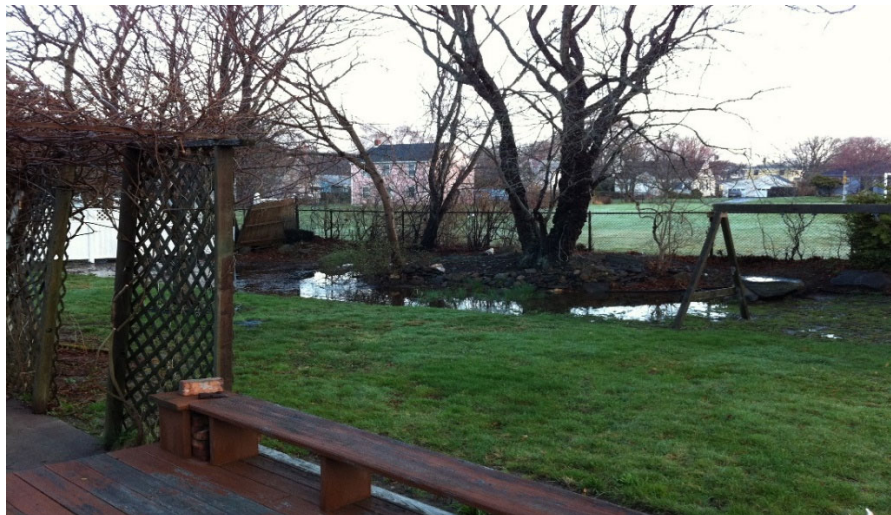


Figure 7. Backyard of 46 Dudley Avenue on April 14, 2020

The photograph in [Figure 8](#) depicts backyard flooding during the March 2nd, 2018 storm event. This event produced a recorded 3.02 inches of rainfall which is comparable to the modeled 2-year storm event (3.4 inches). The simulation model results are 10 inches of maximum flooding depth at this location, which is comparable to the estimated depth of water captured in this photograph. Again, it is possible that the photograph was not taken at exactly the peak flood depth of the storm, so there is room for minor discrepancies between model results and the flood depth in the photograph.



Figure 8. Backyard of 46 Dudley Avenue on March 2, 2020

## Existing Conditions Results

The PCSWMM model of the Dudley Avenue watershed was used to analyze flooding issues observed in the region. The results of the existing conditions model identified and confirmed key areas of expected flooding in the watershed and were used as a baseline for assessing the effectiveness of proposed stormwater infrastructure and green infrastructure improvements. As the water surcharges out of the storm drainage system, it begins to flow over the land surface following the topography. Water that flows down the roadways may re-enter the drainage system downstream if the catch basin structures lower in the system are not surcharging during that model timestep. [Figures 9 through 12](#) depict the maximum simulated flooding depths across the project area for the 1-inch, 2-inch, 2-year, and 10-year frequency storm events. It should be noted that the accumulation and conveyance of stormwater runoff prior to entering model elements (i.e. catch basins, surface channels, etc.) is not shown on the figures.

The results of the existing conditions model showed surface flooding at the following locations for the 1-inch and 2-inch storm events ([Figures 9 and 10](#)):

- The backyards of private residences between 26 Dudley Avenue and 50 Dudley Avenue.
- Runoff from the areas mentioned above eventually inundates the eastern portion of Dudley Avenue near the intersection with Sagamore Street.

For the 2- and 10-year storm event, the results of the existing conditions model showed surface flooding at the following locations ([Figures 11 and 12](#)):

- The backyards of private residences between 26 Dudley Avenue and 50 Dudley Avenue.
- The eastern portion of Dudley Avenue near the intersection with Sagamore Street.
- Dexter Street north and west of the school.
- Runoff originating from the bioretention pond east of the school is designed to overflow its spillway invert when necessary. This runoff travels downhill overland before being captured by infrastructure in the eastern school playground as the original design intended.

The simulated storm events, under the existing conditions, result in runoff water that flows from the eastern portion of the drainage area through the Dudley Avenue backyards influenced by the low topography between the Pell Elementary School boundary and Dudley Avenue. This is primarily due to the significant percentage of the overall watershed area that is located east of Dudley Avenue, and in the school's western grass field, that flows unobstructed towards the private residential backyards on Dudley Avenue. Runoff from approximately 24% (4 acres) of the total watershed area flows unobstructed towards the private residential back yards. The maximum depth of flooding of 11 inches occurs in a low area behind 46 Dudley Avenue for the 10-year storm. This area corresponds to a low point in the topography. Although the flooding in this area is the most severe, there are other areas around the project area that flood, including the private residents east along Dudley Avenue.

The analysis does show a connection forming between the flooding on the southern edge of the school property and the Dudley Avenue backyards ([Figure 11 & 12](#)). However, this connection occurs only after the flood waters in the backyards have already reached peak depth and extent. This indicates that the cause of the connection stems from the water accumulating first in the Dudley Avenue backyards and is not occurring because of water flowing south from the school property.





Figure 9. Simulated Peak Flooding Depths under Existing Conditions (1-inch Storm Event)

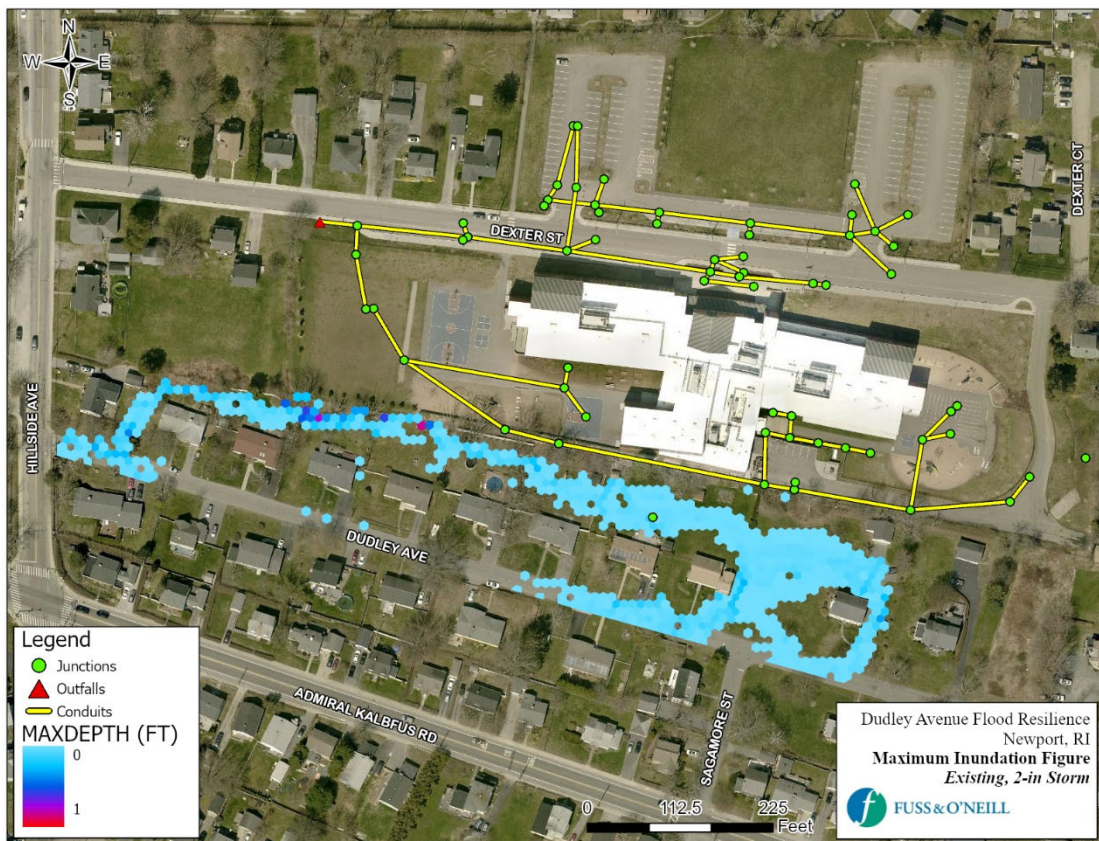


Figure 10. Simulated Peak Flooding Depths under Existing Conditions (2-inch Storm Event)





Figure 11. Simulated Peak Flooding Depths under Existing Conditions (2-Year Storm Event)

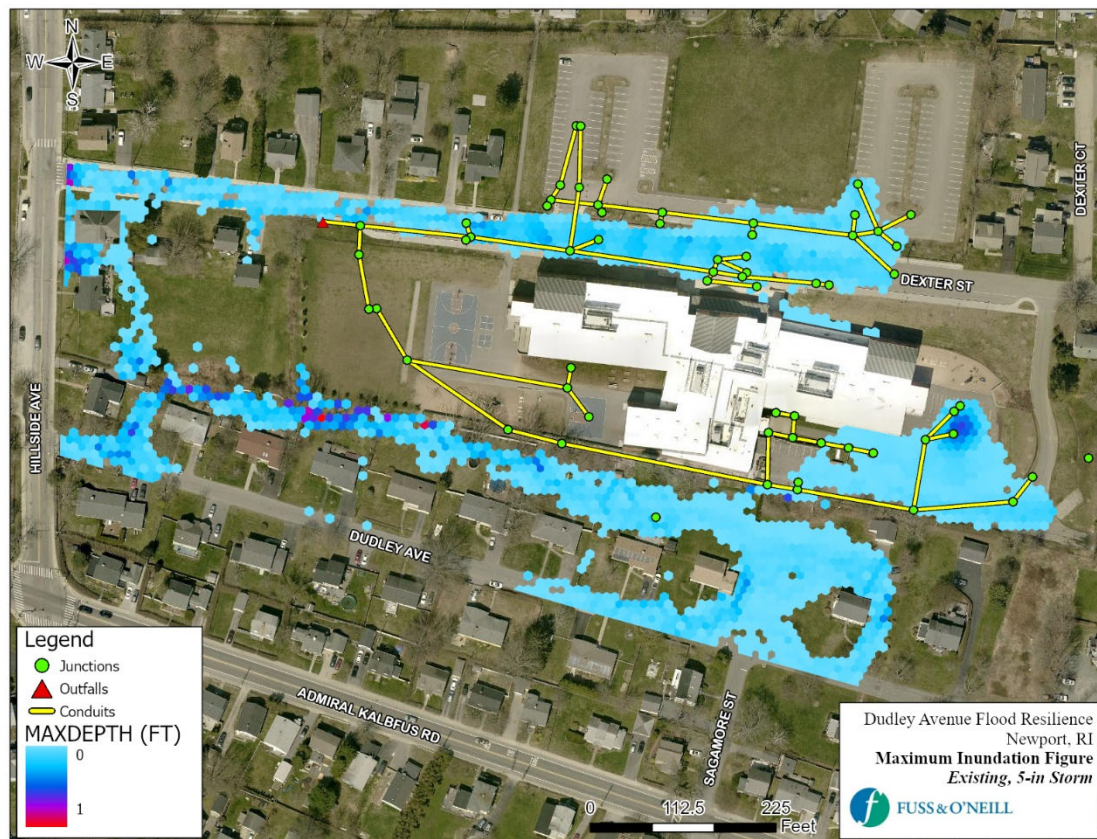


Figure 12. Simulated Peak Flooding Depths under Existing Conditions (10-Year Storm Event)

## 4 Potential Drainage System Improvements

The assessment of stormwater management alternatives, which is discussed below, investigates new stormwater infrastructure improvements, and includes an order of magnitude level cost estimate. The three alternatives were developed with an emphasis placed on addressing nuisance flooding in the backyards of the private residences along Dudley Avenue. All three alternatives focused on installation of new stormwater infrastructure to allow for more efficient capture and retention of stormwater runoff prior to reaching the low-lying backyards. Discharging additional stormwater runoff to the City of Newport's existing stormwater infrastructure on Dudley Avenue was evaluated but not analyzed within the modeled alternatives to prevent increases to peak flows within the existing municipal stormwater system and exacerbating flooding downstream. Additional non-modeled alternative opportunities with considerations of discharging to the existing municipal stormwater systems are described at the end of this section.

Descriptions of the modeled alternatives are included in the sections below. For reference, overview schematics of the three modeled Alternatives are presented as a [Figure 13](#) below. This figure depicts the locations of the two reference points within the backyards where flood depths and duration graphs were calculated. The graphs of the simulated results at these two reference points follow the description of each alternative. It should be noted that the accumulation and conveyance of stormwater runoff prior to entering model elements (i.e. catch basins, surface channels, etc.) is not shown on the figures. The alternatives studied include:

- **Alternative 1 Proposed School Stormwater Infrastructure:** Reduce runoff from western grass field of Pell Elementary School.
- **Alternative 2, Stormwater Infrastructure Along West Main Road Properties:** Reduce runoff from eastern impervious commercial area.
- **Alternative 3, Stormwater Infrastructure on Private Property Along Dudley Avenue:** Reduce peak flood depths by capturing flood water within subsurface infiltration chambers.

Each alternative and corresponding model results are described in section below. Other alternatives considered, but not modeled are described at the end of the section.



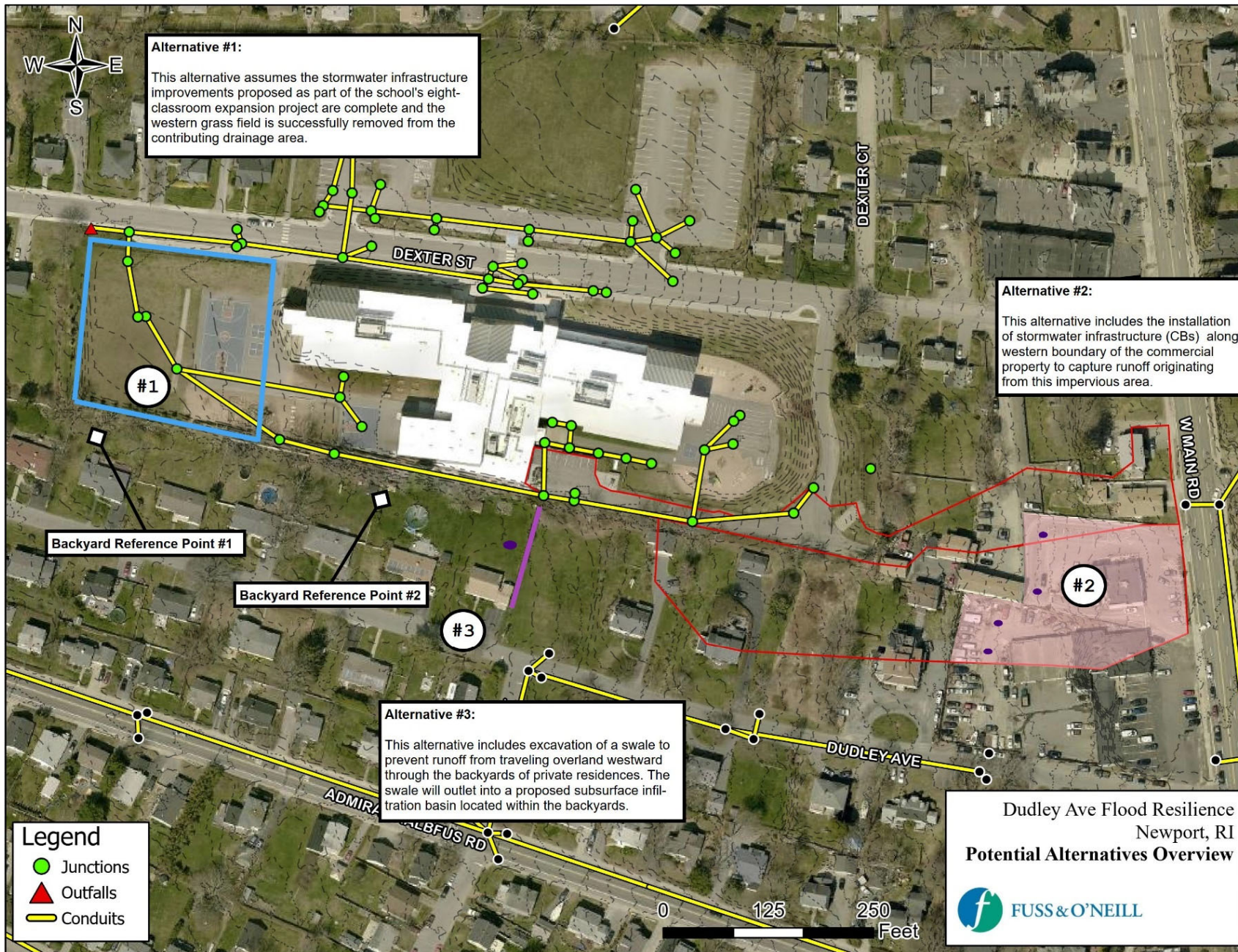


Figure 13. Overview of Alternative Scenarios



## Alternative 1: Proposed School Stormwater Infrastructure

The focus of Alternative 1 was to evaluate the significance of the contribution of stormwater runoff originating from the school's western grass field. This grass field currently slopes towards the southwest, and any precipitation that falls on the field flows southwest downhill, towards the backyards of the private residences. The City of Newport is proposing an eight-classroom expansion project at Claiborne Pell Elementary School. As part of this project, the City is proposing regrading and partial paving of the western grass field, as well as installation of new stormwater infrastructure to capture water accumulating in this area.

Alternative 1 assumes that the stormwater improvements proposed as part of the classroom expansion are complete and successfully capture all stormwater runoff originating from the field, as designed, thereby removing the school's western grass field from the drainage area contributing stormwater runoff to the private residences along Dudley Avenue.

The results of the Alternative 1 PCSWMM analyses indicate that the removal of the western grass field from the contributing drainage area produced no significant reduction in maximum flood inundation extent, depth, or duration within the residential backyards along Dudley Avenue for any of the four modeled storm events.

The maximum extents of surface flooding for each of the four modeled storm events are provided as Figures 14-17.



Figure 14. Simulated Peak Flooding Depths under Alternative 1 (1-inch Storm Event)



Figure 15. Simulated Peak Flooding Depths under Alternative 1 (2-inch Storm Event)



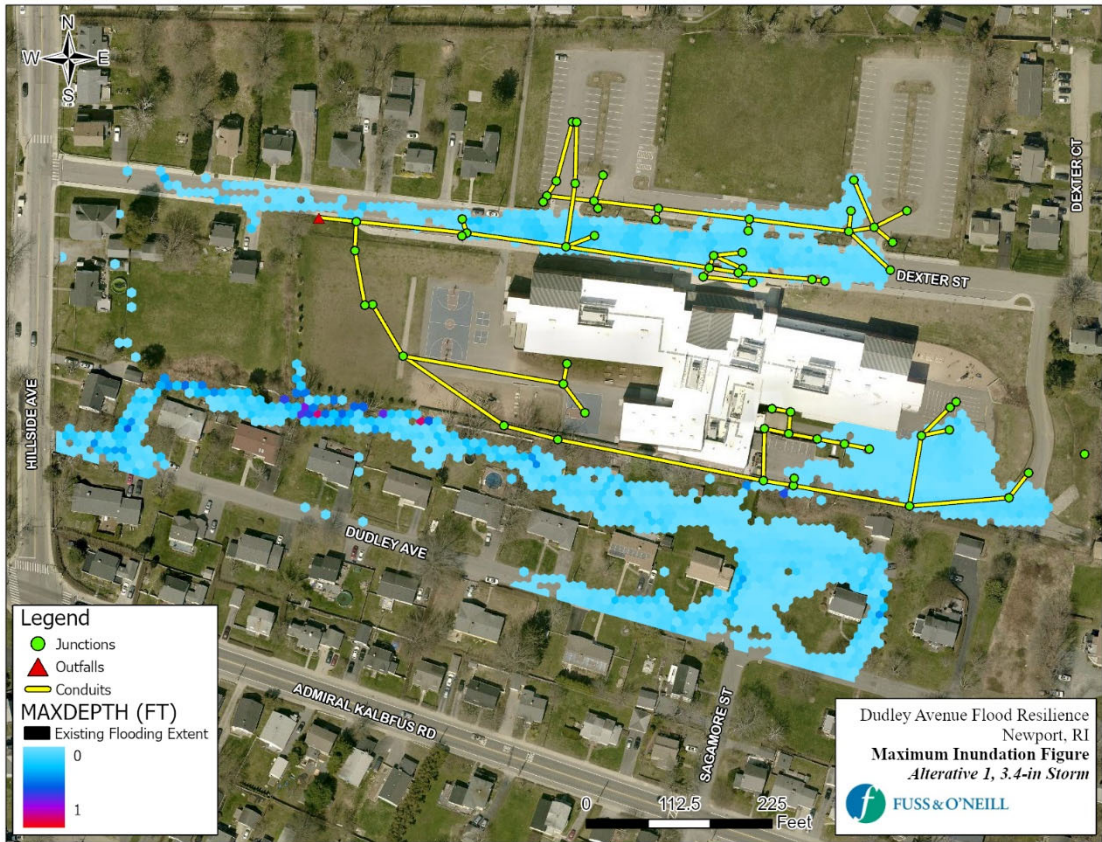


Figure 16. Simulated Peak Flooding Depths under Alternative 1 (2-Year Storm Event)



Figure 17. Simulated Peak Flooding Depths under Alternative 1 (10-Year Storm Event)



## Alternative 2: Stormwater Infrastructure Along West Main Road Properties

The focus of Alternative 2 was to evaluate the significance of the contribution of stormwater runoff originating from the eastern impervious commercial parking area along West Main Road. This area currently slopes towards the southwest, and any precipitation that falls on the pavement flows southwest downhill, towards the backyards of the private residences along Dudley Avenue. Alternative 2 includes the installation of stormwater infrastructure (catch basins) along the western boundary of the impervious commercial area to capture runoff before it reaches the backyards. Additionally, Alternative 2 includes the school stormwater infrastructure improvements proposed within Alternative 1.

In general, the results of the Alternative 2 PCSWMM analyses indicate that the proposed stormwater infrastructure improvements help to reduce stormwater flooding in the Dudley Avenue backyard areas under the more frequent, lower depth storm events. Summaries of the simulated flood reductions as a result of the proposed improvements are as follows:

- Reduction of maximum surface flood extent within the backyards of private residences between 26 Dudley Avenue and 50 Dudley Avenue under the 1-inch and 2-inch storm events.
- Minor reductions in flood depth and durations at both reference locations within the Dudley Avenue backyards under the 1-inch and 2-inch storm events.
- The improvements had diminishing returns under larger storms as there was no significant reduction in maximum flood extent, depth, or duration within the backyards under the 3.4-inch and 5-inch storm events.

The maximum extents of surface flooding for each of the four modeled storm events are shown in [Figures 18-21](#) below.

This alternative faces a unique construction challenge. It should be noted that these improvements are not located within the City of Newport and are on commercial properties in Middletown, RI. Coordination would be required with the Town of Middletown, RI for any further consideration of implementation.



Figure 18. Simulated Peak Flooding Depths under Alternative 2 (1-inch Storm Event)



Figure 19. Simulated Peak Flooding Depths under Alternative 2 (2-inch Storm Event)





Figure 20. Simulated Peak Flooding Depths under Alternative 2 (2-Year Storm Event)



Figure 21. Simulated Peak Flooding Depths under Alternative 2 (10-Year Storm Event)



## Alternative 3: Stormwater Infrastructure on Private Property Along Dudley Avenue

The focus of Alternative 3 was to evaluate the potential to reduce stormwater depths and durations by installing storm infrastructure directly within the backyards. Specifically, Alternative 3 includes the excavation of a small swale to prevent the propagation of surface flooding from traveling west across the low-lying areas within the backyards. The swale outlets into a subsurface infiltration chamber that retains approximately 2,200 ft<sup>3</sup> of stormwater. The subsurface infiltration chamber was sized based on physical construction constraints of a backyard installation as well as the constraints of the underlying groundwater table elevations. The subsurface storage aims to reduce the maximum flood extent and peak flood depths currently experienced by holding a portion of the total stormwater runoff volume. In addition, Alternative 3 includes the school stormwater infrastructure improvements proposed within Alternative 1.

The results of the Alternative 3 PCSWMM analyses indicate that the proposed stormwater improvements within the backyards would marginally reduce maximum stormwater flooding extents and depths under storm events greater than the 1-inch storm. Summaries of the simulated flood reductions as a result of the proposed improvements are as follows:

- Reduction of maximum surface flood extent, depth, and duration within the backyards of private residences between 26 Dudley Avenue and 50 Dudley Avenue under the 1-inch storm event.
- Minor reductions in flood depth and durations at both reference locations within the backyards under the 2-inch, 3.4-inch, and 5-in storm events.
- The improvements had diminishing returns under larger storms. There was no significant reduction in maximum flood extent within the backyards under the 2-inch, 3.4-inch, or 5-inch storm events.
- The subsurface infiltration basin is only filled to capacity under the 3.4- and 5-inch storm events. While increasing the total maximum volume of the underground chamber could potentially result in greater reductions in flood extents, depths, and durations during these larger storm events, the results would likely be minimal under the smaller, more frequent storms as the capacity of the underground chamber is not the limiting factor during these modeled storm events. A variety of conveyance system configurations (swales lengths and catch basin placements) were investigated within this alternative, producing less substantial results. A larger, and much more extensive conveyance system would need to be designed to collect and convey the flood water to fully utilize the capacity of the subsurface infiltration system under the more frequent storm events. The design of such a system would need to include the evaluation of a number of hydrologic and subsurface factors and constraints that are outside of this current scope of this project.

The maximum extents of surface flooding for each of the modeled storm events are provided as [Figures 22-25](#) below.



Figure 22. Simulated Peak Flooding Depths under Alternative 3 (1-inch Storm Event)



Figure 23. Simulated Peak Flooding Depths under Alternative 3 (2-inch Storm Event)





Figure 24. Simulated Peak Flooding Depths under Alternative 3 (2-Year Storm Event)



Figure 25. Simulated Peak Flooding Depths under Alternative 3 (10-Year Storm Event)

Depths and durations of flooding at the two reference locations (depicted on Figure 13) within the backyards for all existing conditions and proposed alternative model runs are provided as Figures 26-33.

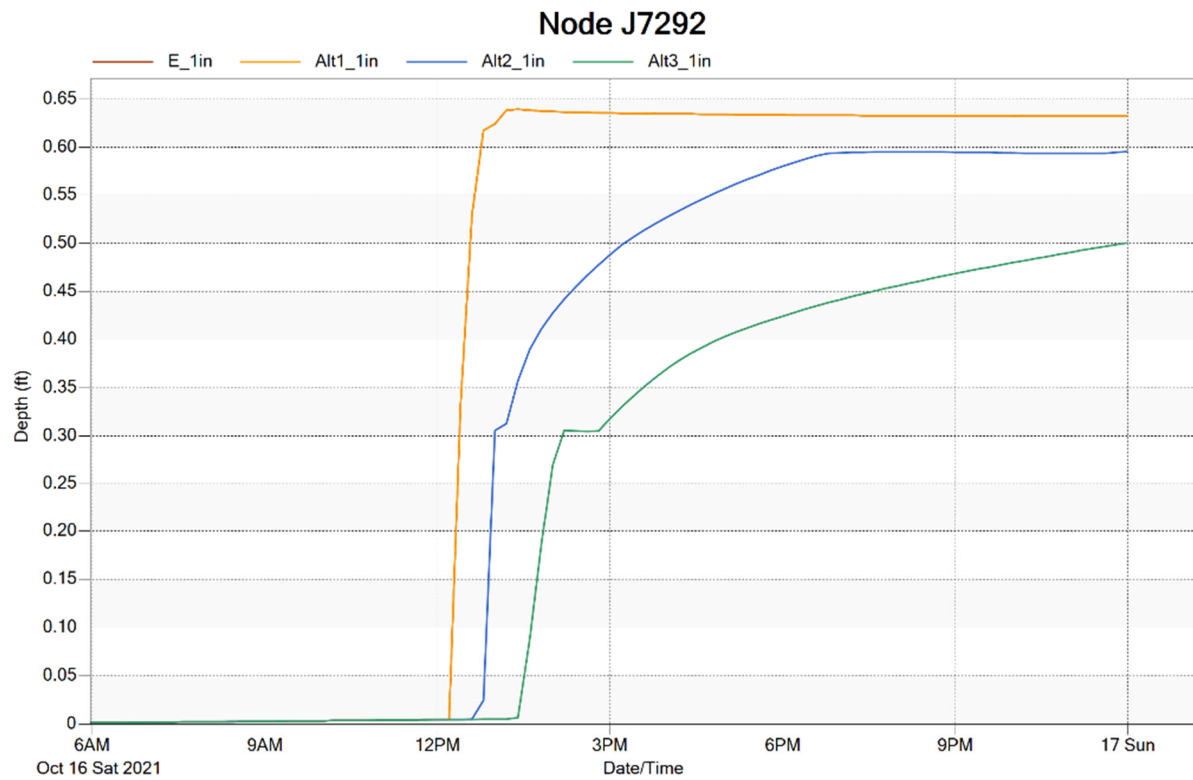


Figure 26. Flood Depths and Durations at Reference Point #1 (1-inch Storm Event)

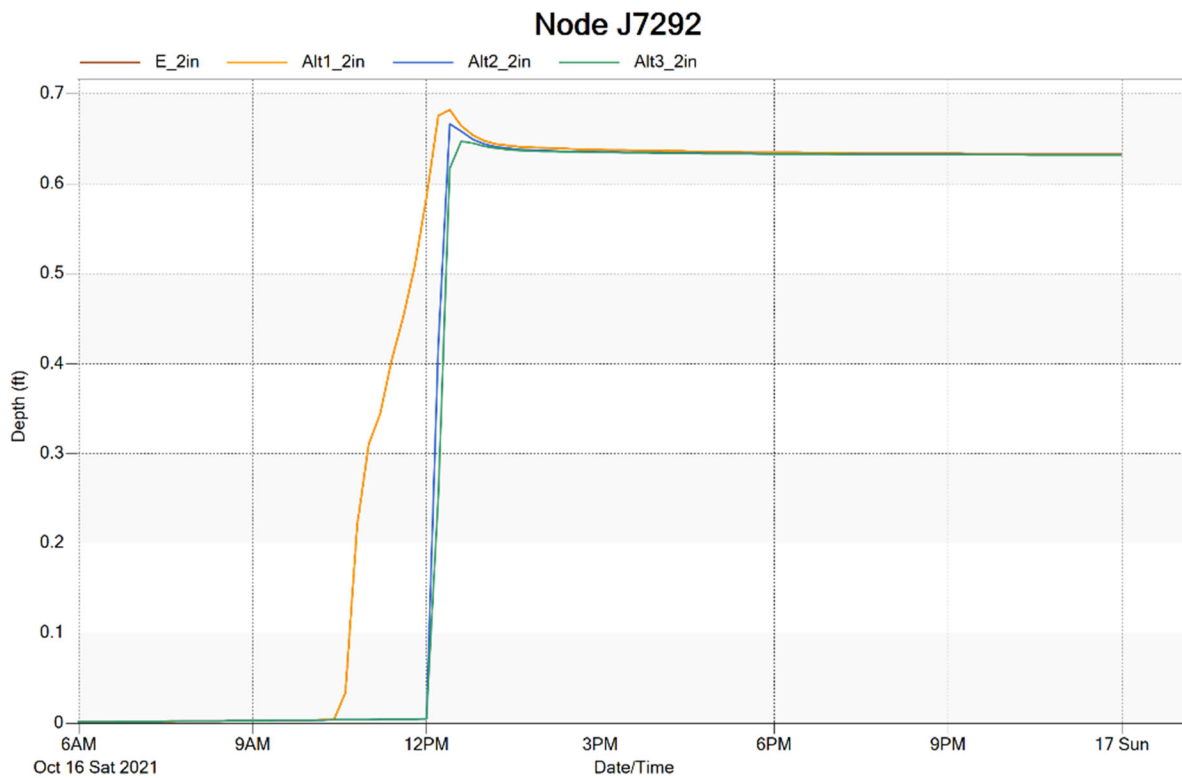


Figure 27. Flood Depths and Durations at Reference Point #1 (2-inch Storm Event)



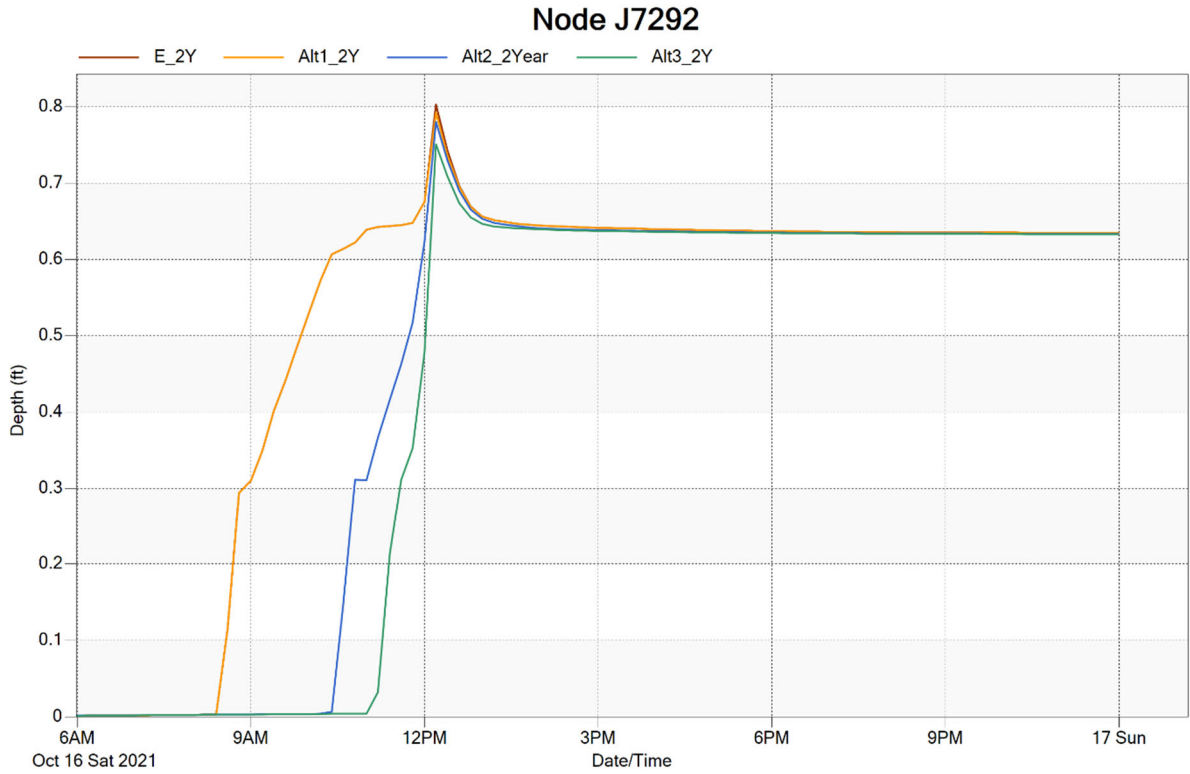


Figure 28. Flood Depths and Durations at Reference Point #1 (2-Year Storm Event)

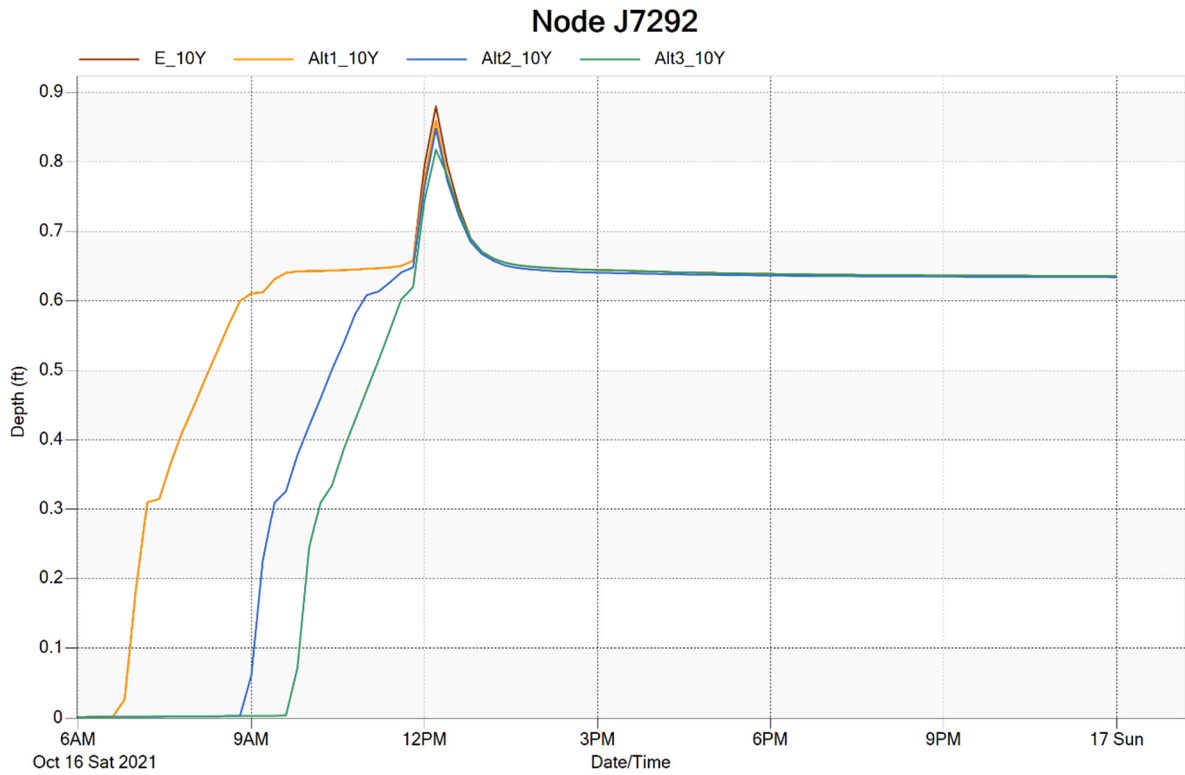


Figure 29. Flood Depths and Durations at Reference Point #1 (10-Year Storm Event)

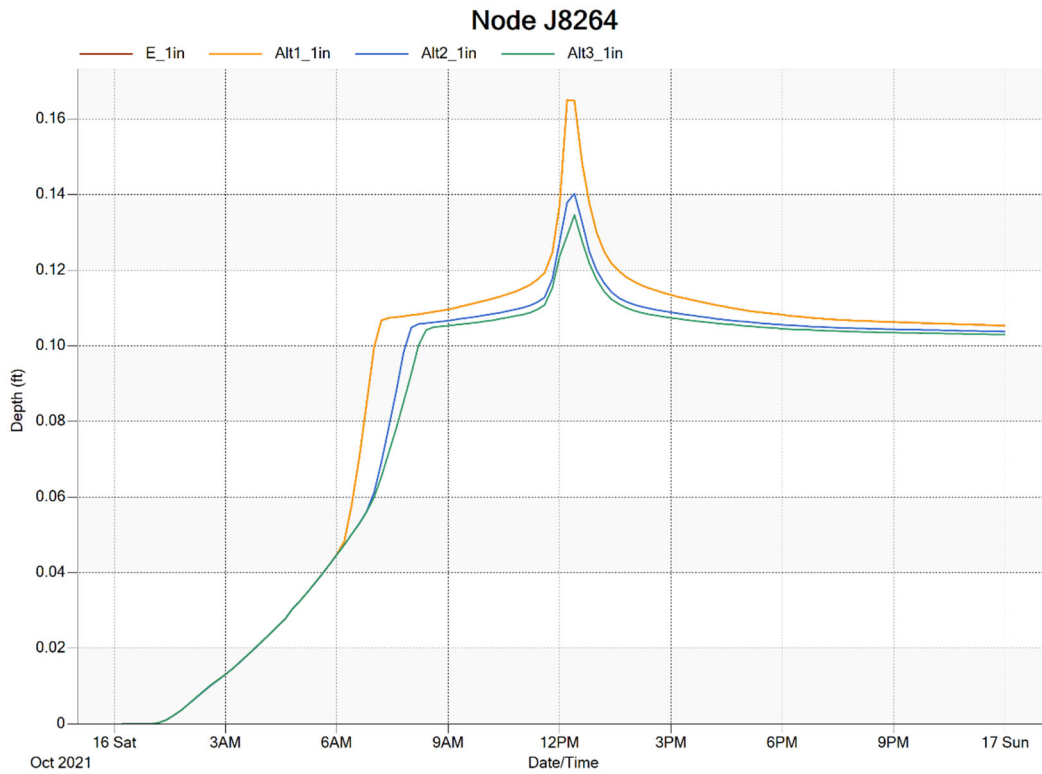


Figure 30. Flood Depths and Durations at Reference Point #2 (1-inch Storm Event)

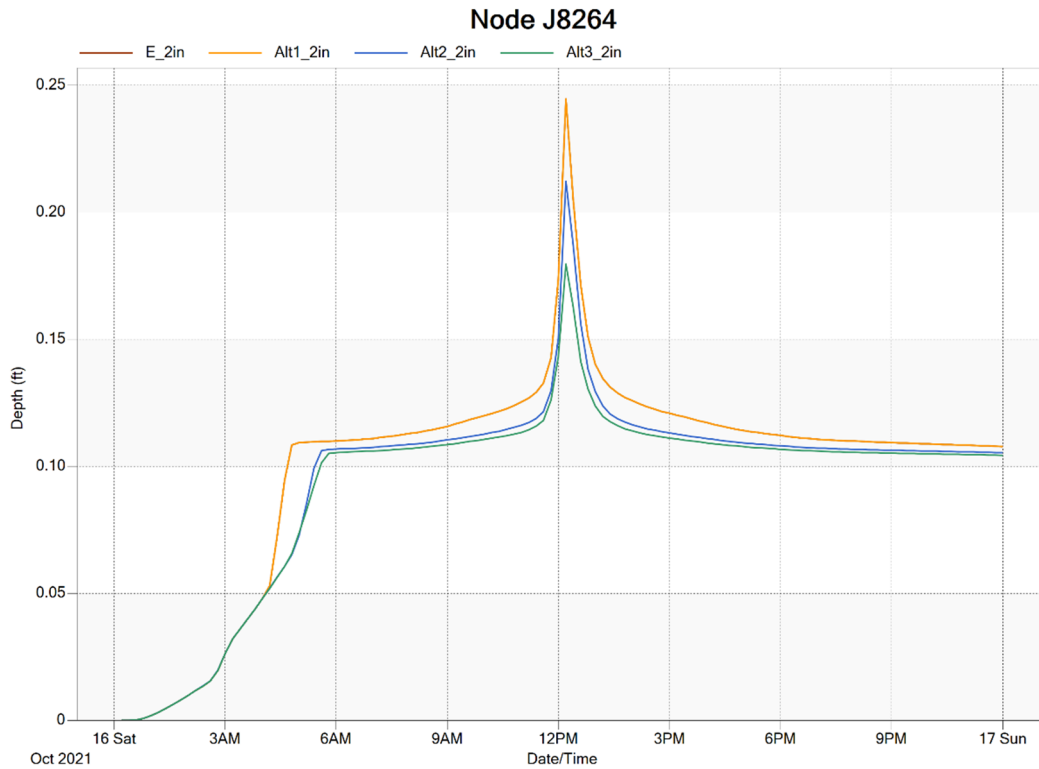


Figure 31 Flood Depths and Durations at Reference Point #2 (2-inch Storm Event)



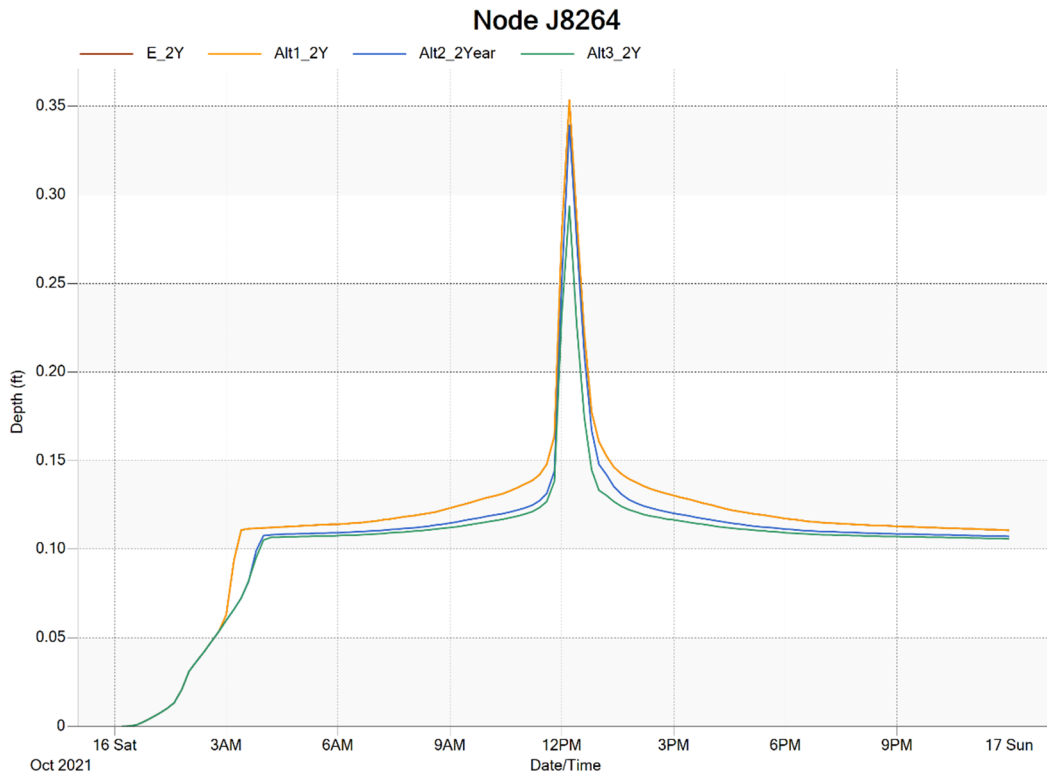


Figure 32. Flood Depths and Durations at Reference Point #2 (2-Year Storm Event)

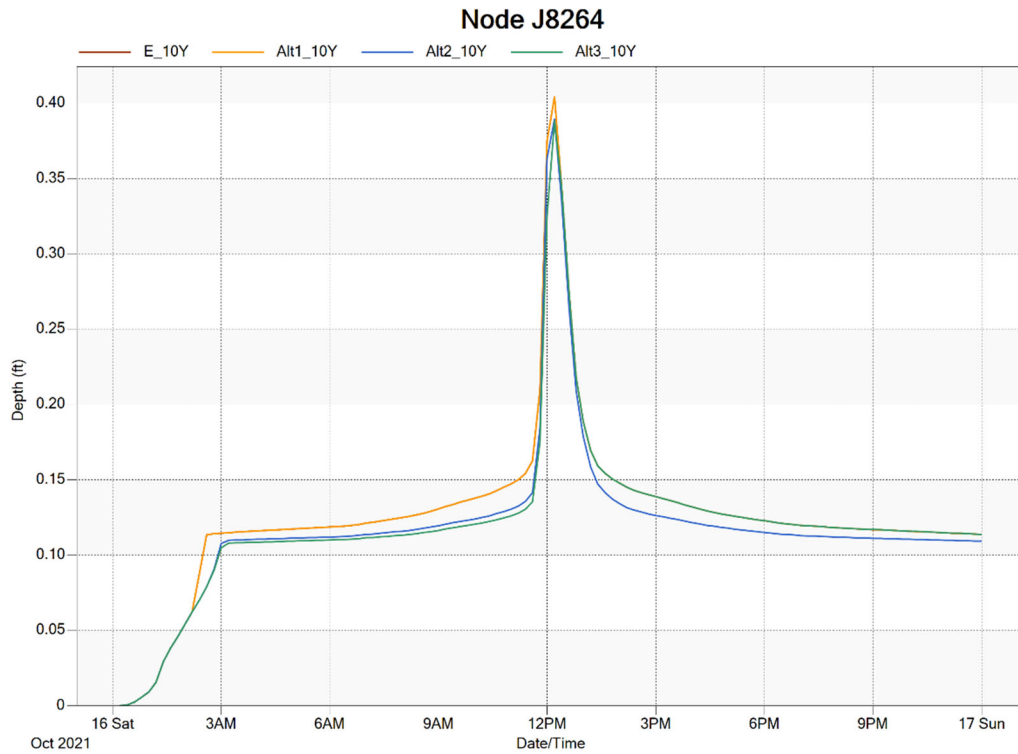


Figure 33. Flood Depths and Durations at Reference Point #2 (10-Year Storm Event)

## Other Alternatives Considered

Implementation of stormwater infrastructure improvements within the backyards that connect directly into the existing municipal stormwater system were initially considered as potential project alternatives but not carried forward into the modelling analysis. Additional alternatives could be explored such as installing catch basins in low lying areas within the Dudley Avenue backyards to drain and convey flood water to the existing municipal system. Additionally, the proposed Alternative 3 could be modified by incorporating an overflow structure within the subsurface infiltration chamber that would outlet excess flows to the existing municipal system at the intersection of Dudley Avenue and Sagamore Street, thereby increasing the structure's overall capacity.

In order to fully evaluate and model the effects of these alternatives, a much larger scale hydraulic analysis effort would be necessary which is outside of the scope of this project. This analysis would need to include the entirety of the municipal stormwater system in order to assure that the added flow contributions from the proposed alternatives would not significantly impact peak flows and further exacerbate flooding issues occurring in other parts of the system. A hydraulic analysis effort of this magnitude would likely cost on the order of \$250,000-\$500,000.

In addition to the alternatives examined in this study, there are opportunities for private homeowners along Dudley Avenue to implement small-scale green infrastructure practices, like swales or raingardens, on their own properties. These small-scale practices have the potential to produce localized reductions to flood depths and extents. However, these practices would produce more significant beneficial cumulative effects when implemented together with the proposed alternatives mentioned above.

## Summary of Costs

An Order of Magnitude level opinion of cost has been prepared for the proposed improvements as part of Alternatives 2 and 3. The improvements included in Alternative 1 are part of the City's proposed eight-class expansion project that is still under development. As a result, the total cost for Alternative 1 is not included within this report. The opinion of costs are provided in the table below; detailed back up documentation is provided in *Appendix B*. A more detailed opinion of cost should be developed as part of a future design which will allow issues such as utility conflicts and constructability to be better assessed.

**Table 2: Stormwater Infrastructure Improvements  
Order of Magnitude Opinion of Cost\***

Description	Estimated Cost	-30%	+50%
Alternative #1	Not Calculated	Not Calculated	Not Calculated
Alternative #2	\$240,000	\$160,000	\$350,000
Alternative #3	\$50,000	\$30,000	\$70,000



## 5 Conclusion

The results of the existing conditions analysis confirmed surface flooding in locations that corresponds with community feedback. However, the existing conditions analysis suggest that stormwater runoff causing flooding under the 1- and 2-inch storms does not originate from the school property, but rather originate from the eastern portions of the drainage area without stormwater infrastructure. Under the higher intensity 3.4- and 5-inch storms, while the existing conditions analysis shows a connection forming between the flooding on the southern school property and the flooding in the backyards, this connection occurs after the flood water in the backyards of the private residences has already reached its peak depth and extent. This would suggest that the connection is the result of flood water accumulating in the backyards rather than the source of the flooding.

The results of the proposed alternative analyses have been compared to the existing conditions results to evaluate the response and efficiency of the proposed stormwater improvements. The results of the proposed conditions model show a reduction in peak flood depths and reduced durations of flooding in the backyards of the private residential properties on Dudley Avenues. The results also show that there would be no meaningful improvements in areas further upland in the watershed.

In addition to the alternatives examined in this study, there are opportunities for private homeowners along Dudley Avenue to implement small-scale green infrastructure practices, like swales or raingardens, on their own properties. These small-scale practices have the potential to produce localized reductions to flood depths and extents. However, these practices would produce more significant beneficial cumulative effects when implemented together with the proposed alternatives mentioned above.

SWMM Model Input Data



[TITLE]  
 ;;Project Title/Notes  
 Existing Conditions Basemodel for Claiborne Pell Elementary School Stormwater Drainage System and

[OPTIONS]  
 ;;Option Value  
 FLOW\_UNITS CFS  
 INFILTRATION MODIFIED\_GREEN\_AMPT  
 FLOW\_ROUTING DYNWAVE  
 LINK\_OFFSETS DEPTH  
 MIN\_SLOPE 0  
 ALLOW\_PONDING NO  
 SKIP\_STEADY\_STATE NO

START\_DATE 10/16/2021  
 START\_TIME 00:00:00  
 REPORT\_START\_DATE 10/16/2021  
 REPORT\_START\_TIME 00:00:00  
 END\_DATE 10/17/2021  
 END\_TIME 00:00:00  
 SWEEP\_START 01/01  
 SWEEP\_END 12/31  
 DRY\_DAYS 0  
 REPORT\_STEP 00:12:00  
 WET\_STEP 00:12:00  
 DRY\_STEP 00:12:00  
 ROUTING\_STEP 0.5  
 RULE\_STEP 00:00:00

INERTIAL\_DAMPING FULL  
 NORMAL\_FLOW\_LIMITED BOTH  
 FORCE\_MAIN\_EQUATION H-W  
 VARIABLE\_STEP 0.75  
 LENGTHENING\_STEP 0  
 MIN\_SURFAREA 1  
 MAX\_TRIALS 8  
 HEAD\_TOLERANCE 0.005  
 SYS\_FLOW\_TOL 5  
 LAT\_FLOW\_TOL 5  
 MINIMUM\_STEP 0.5  
 THREADS 4

[EVAPORATION]  
 ;;Data Source Parameters  
 ;;-----  
 CONSTANT 0.0  
 DRY\_ONLY NO

[RAINGAGES]  
 ;;Name Format Interval SCF Source  
 ;;-----  
 10Year\_5.03in CUMULATIVE 0:06 1.0 TIMESERIES 10Year\_5.03in  
 1Year\_2.86 CUMULATIVE 0:06 1.0 TIMESERIES 1Year\_2.86  
 2Year\_3.4in CUMULATIVE 0:06 1.0 TIMESERIES SCS\_Type\_III\_3.4in  
 SCS\_Type\_III\_1in CUMULATIVE 0:06 1.0 TIMESERIES SCS\_Type\_III\_1in  
 SCS\_Type\_III\_2in CUMULATIVE 0:06 1.0 TIMESERIES SCS\_Type\_III\_2in

[SUBCATCHMENTS]  
 ;;Name Rain Gage Outlet Area %Imperv Width %Slope CurbLen S  
 ;;-----  
 SBioretentionPond SCS\_Type\_III\_1in BioretentionPond 1.46548 35.45 166.447 6.156 0  
 SCB01 SCS\_Type\_III\_1in CB01 0.468743 84.17 90.378 2.83 0  
 SCB02 SCS\_Type\_III\_1in CB02 0.370313 20.52 44.241 6.08 0  
 SCB03 SCS\_Type\_III\_1in CB03 0.238517 84.18 56.639 5.863 0

SCB04	SCS_Type_III_lin	CB04	0.17782	63.34	24.211	4.026	0
SCB05	SCS_Type_III_lin	CB05	0.38726	40.74	47.965	4.485	0
SCB06	SCS_Type_III_lin	CB06	0.535731	27.58	66.752	4.391	0
SCB07	SCS_Type_III_lin	CB07	0.088744	99.54	53.554	3.322	0
SCB08	SCS_Type_III_lin	CB08	0.86883	49.08	88.116	6.568	0
SCB09	SCS_Type_III_lin	CB09	1.012711	62.18	77.21	6.366	0
SCB10	SCS_Type_III_lin	CB10	1.808208	78.61	145.656	6.118	0
SCB11	SCS_Type_III_lin	CB11	0.173435	87.75	41.764	4.362	0
SCB12	SCS_Type_III_lin	CB12	0.011985	70.8	10.559	3.541	0
SCB13	SCS_Type_III_lin	CB13	0.74901	25.67	167.373	10.505	0
SCB14	SCS_Type_III_lin	CB14	0.291766	32.3	109.827	7.915	0
SCB15	SCS_Type_III_lin	CB15	0.392417	22.44	53.584	8.12	0
SCB16	SCS_Type_III_lin	CB16	0.915073	49.128	53.101	5.937	0
SCB18	SCS_Type_III_lin	CB18	0.158004	81.77	58.565	2.959	0
SCB19	SCS_Type_III_lin	CB19	0.407989	82.68	73.888	3.917	0
SCB20	SCS_Type_III_lin	CB20	0.424693	53.99	78.415	5.399	0
SCB21	SCS_Type_III_lin	CB21	0.401173	58.79	59.696	3.899	0
SCB22	SCS_Type_III_lin	CB22	0.128954	40.24	20.793	4.924	0
SCB23	SCS_Type_III_lin	CB23	0.084257	100	57.093	4.912	0
SCB24	SCS_Type_III_lin	CB24	0.080453	78.48	76.245	2.78	0
SCB25	SCS_Type_III_lin	CB25	0.010208	100	27.218	0.279	0
SCB26	SCS_Type_III_lin	CB26	0.012529	100	20.7	0.987	0
SCB27	SCS_Type_III_lin	CB27	0.221182	99.31	74.229	3.575	0
SCB28	SCS_Type_III_lin	CB28	0.014923	100	33.814	1.485	0
SCB29	SCS_Type_III_lin	CB29	0.049416	100	40.352	1.716	0
SCB30	SCS_Type_III_lin	CB30	0.059407	79.23	18.359	1.216	0
SCB31	SCS_Type_III_lin	CB31	0.111778	26.34	33.351	3.911	0
SDMH10	SCS_Type_III_lin	DMH10	0.217367	97.39	45.685	0.367	0
SDMH14	SCS_Type_III_lin	DMH14	0.288943	3.03	70.289	5.46	0
SDMH15	SCS_Type_III_lin	DMH15	0.388265	4.44	116.698	4.341	0
SDMH23	SCS_Type_III_lin	DMH23	0.209088	96.79	33.843	0.1	0
SINFIL01	SCS_Type_III_lin	INFIL01	0.698785	96.09	111.951	9.602	0
SRunOff01	SCS_Type_III_lin	RunOff01	0.769005	1.84	143.189	5.869	0
SRunOff02	SCS_Type_III_lin	RunOff02	1.811274	65.912	125.86	5.917	0
SRunOff03	SCS_Type_III_lin	RunOff03	0.626662	7.31	106.835	7.511	0
SRunOff04	SCS_Type_III_lin	RunOff04	0.379143	9.93	94.445	9.838	0
SRunOff05	SCS_Type_III_lin	RunOff05	0.355106	4.22	138.07	9.004	0

[SUBAREAS]

;;Subcatchment	N-Imperv	N-Perv	S-Imperv	S-Perv	PctZero	RouteTo	PctRouted
;;-----	-----	-----	-----	-----	-----	-----	-----
SBioretentionPond	0.01	0.1	0.05	0.05	20.211	OUTLET	
SCB01	0.01	0.1	0.05	0.05	0	OUTLET	
SCB02	0.01	0.1	0.05	0.05	0	OUTLET	
SCB03	0.01	0.1	0.05	0.05	8.337	OUTLET	
SCB04	0.01	0.1	0.05	0.05	0	OUTLET	
SCB05	0.01	0.1	0.05	0.05	0	OUTLET	
SCB06	0.01	0.1	0.05	0.05	0	OUTLET	
SCB07	0.01	0.1	0.05	0.05	0	OUTLET	
SCB08	0.01	0.1	0.05	0.05	19.628	OUTLET	
SCB09	0.01	0.1	0.05	0.05	17.057	OUTLET	
SCB10	0.01	0.1	0.05	0.05	23.974	OUTLET	
SCB11	0.01	0.1	0.05	0.05	0	OUTLET	
SCB12	0.01	0.1	0.05	0.05	0	OUTLET	
SCB13	0.01	0.1	0.05	0.05	0	OUTLET	
SCB14	0.01	0.1	0.05	0.05	2.196	OUTLET	
SCB15	0.01	0.1	0.05	0.05	37.388	OUTLET	
SCB16	0.01	0.1	0.05	0.05	16.897	OUTLET	
SCB18	0.01	0.1	0.05	0.05	0.683	OUTLET	
SCB19	0.01	0.1	0.05	0.05	0	OUTLET	
SCB20	0.01	0.1	0.05	0.05	5.565	OUTLET	
SCB21	0.01	0.1	0.05	0.05	11.544	OUTLET	
SCB22	0.01	0.1	0.05	0.05	0	OUTLET	
SCB23	0.01	0.1	0.05	0.05	0	OUTLET	



SCB24	0.01	0.1	0.05	0.05	0	OUTLET
SCB25	0.01	0.1	0.05	0.05	2.575	OUTLET
SCB26	0.01	0.1	0.05	0.05	0	OUTLET
SCB27	0.01	0.1	0.05	0.05	0	OUTLET
SCB28	0.01	0.1	0.05	0.05	0	OUTLET
SCB29	0.01	0.1	0.05	0.05	0	OUTLET
SCB30	0.01	0.1	0.05	0.05	0	OUTLET
SCB31	0.01	0.1	0.05	0.05	0	OUTLET
SDMH10	0.01	0.1	0.05	0.05	0	OUTLET
SDMH14	0.01	0.1	0.05	0.05	0	OUTLET
SDMH15	0.01	0.1	0.05	0.05	8.863	OUTLET
SDMH23	0.01	0.1	0.05	0.05	0	OUTLET
SINFIL01	0.01	0.1	0.05	0.05	0	OUTLET
SRunOff01	0.01	0.1	0.05	0.05	0	OUTLET
SRunOff02	0.01	0.1	0.05	0.05	24.594	OUTLET
SRunOff03	0.01	0.1	0.05	0.05	42.74	OUTLET
SRunOff04	0.01	0.1	0.05	0.05	23.809	OUTLET
SRunOff05	0.01	0.1	0.05	0.05	0	OUTLET

[INFILTRATION]

;;Subcatchment	Suction	Ksat	IMD
SBioretenionPond	6.57	0.27	0.171
SCB01	4.544	0.804	0.239
SCB02	4.356	0.853	0.245
SCB03	4.33	0.86	0.246
SCB04	4.586	0.792	0.237
SCB05	4.344	0.856	0.246
SCB06	4.402	0.841	0.244
SCB07	4.679	0.768	0.234
SCB08	6.568	0.271	0.171
SCB09	6.558	0.273	0.171
SCB10	6.56	0.273	0.171
SCB11	4.33	0.86	0.246
SCB12	4.33	0.86	0.246
SCB13	4.8	0.736	0.23
SCB14	4.33	0.86	0.246
SCB15	6.404	0.314	0.177
SCB16	5.952	0.433	0.192
SCB18	4.33	0.86	0.246
SCB19	5.928	0.439	0.193
SCB20	5.116	0.653	0.22
SCB21	5.114	0.653	0.22
SCB22	5.923	0.441	0.193
SCB23	4.33	0.86	0.246
SCB24	4.33	0.86	0.246
SCB25	4.33	0.86	0.246
SCB26	4.33	0.86	0.246
SCB27	4.33	0.86	0.246
SCB28	4.33	0.86	0.246
SCB29	4.33	0.86	0.246
SCB30	4.33	0.86	0.246
SCB31	4.33	0.86	0.246
SDMH10	4.33	0.86	0.246
SDMH14	4.33	0.86	0.246
SDMH15	4.408	0.839	0.243
SDMH23	4.33	0.86	0.246
SINFIL01	4.33	0.86	0.246
SRunOff01	5.144	0.646	0.219
SRunOff02	6.545	0.277	0.172
SRunOff03	6.228	0.36	0.182
SRunOff04	5.929	0.439	0.192
SRunOff05	6.029	0.412	0.189

[JUNCTIONS]

;;Name	Elevation	MaxDepth	InitDepth	SurDepth	Aponded
CB01	87.2	3	0	30	0
CB02	88.5	2.9	0	30	0
CB03	86.9	3.4	0	30	0
CB04	88.9	3.2	0	30	0
CB05	90.1	2.6	0	30	0
CB06	92.1	2.6	0	30	0
CB07	97.3	3	0	30	0
CB08	98.1	3.4	0	30	0
CB09	97.1	3.3	0	30	0
CB10	97.1	3.6	0	30	0

.....

Too many junction entities (11240 in total).

[OUTFALLS]

;;Name	Elevation	Type	Stage Data	Gated	Route To
OF1	76.3	FREE		NO	
OF10	109.56	FREE		NO	
OF100	81.96	FREE		NO	
OF101	81.92	FREE		NO	
OF102	81.39	FREE		NO	
OF103	81.31	FREE		NO	
OF104	81.2	FREE		NO	
OF105	80.84	FREE		NO	
OF106	80.95	FREE		NO	
OF107	81.17	FREE		NO	
OF108	80.78	FREE		NO	
OF109	80.33	FREE		NO	
OF11	106.14	FREE		NO	
OF110	80.5	FREE		NO	
OF111	80.13	FREE		NO	
OF112	80.24	FREE		NO	
OF113	80.15	FREE		NO	
OF114	79.56	FREE		NO	
OF115	79.74	FREE		NO	
OF116	79.93	FREE		NO	
OF117	79.42	FREE		NO	
OF118	79.37	FREE		NO	
OF119	79.44	FREE		NO	
OF12	106.36	FREE		NO	
OF120	79.1	FREE		NO	
OF121	79.15	FREE		NO	
OF122	78.94	FREE		NO	
OF123	78.8	FREE		NO	
OF124	78.52	FREE		NO	
OF125	78.48	FREE		NO	
OF126	78.42	FREE		NO	
OF127	78.39	FREE		NO	
OF128	78.39	FREE		NO	
OF129	78.46	FREE		NO	
OF13	104.5	FREE		NO	
OF130	78.12	FREE		NO	
OF131	78.06	FREE		NO	
OF132	78.26	FREE		NO	
OF133	78.44	FREE		NO	
OF134	78.26	FREE		NO	
OF135	78.12	FREE		NO	
OF136	78.35	FREE		NO	
OF137	77.76	FREE		NO	
OF138	78.12	FREE		NO	
OF139	77.6	FREE		NO	



OF14	104.98	FREE	NO
OF15	105.41	FREE	NO
OF16	105.71	FREE	NO
OF17	109.67	FREE	NO
OF18	104.37	FREE	NO
OF19	103.82	FREE	NO
OF2	108.59	FREE	NO
OF20	103.9	FREE	NO
OF21	103.08	FREE	NO
OF22	103.72	FREE	NO
OF23	109.71	FREE	NO
OF24	101.92	FREE	NO
OF25	102.33	FREE	NO
OF26	100.52	FREE	NO
OF27	100.87	FREE	NO
OF28	100.15	FREE	NO
OF29	99.51	FREE	NO
OF3	108.36	FREE	NO
OF30	99.73	FREE	NO
OF31	98.16	FREE	NO
OF32	98.45	FREE	NO
OF33	98.82	FREE	NO
OF34	97.57	FREE	NO
OF35	111.02	FREE	NO
OF36	96.81	FREE	NO
OF37	97.47	FREE	NO
OF38	97.63	FREE	NO
OF39	96.55	FREE	NO
OF4	108.48	FREE	NO
OF40	111.22	FREE	NO
OF41	95.9	FREE	NO
OF42	95.54	FREE	NO
OF43	95.01	FREE	NO
OF44	111.47	FREE	NO
OF45	95.08	FREE	NO
OF46	94.16	FREE	NO
OF47	93.37	FREE	NO
OF48	93.7	FREE	NO
OF49	94.13	FREE	NO
OF5	109.44	FREE	NO
OF50	111.87	FREE	NO
OF51	93.14	FREE	NO
OF52	92.43	FREE	NO
OF53	92.82	FREE	NO
OF54	91.89	FREE	NO
OF55	112.1	FREE	NO
OF56	91.34	FREE	NO
OF57	91.93	FREE	NO
OF58	112.26	FREE	NO
OF59	90.65	FREE	NO
OF6	106.64	FREE	NO
OF60	89.46	FREE	NO
OF61	90.12	FREE	NO
OF62	90.42	FREE	NO
OF63	112.56	FREE	NO
OF64	88.52	FREE	NO
OF65	88.91	FREE	NO
OF66	112.59	FREE	NO
OF67	112.88	FREE	NO
OF68	87.03	FREE	NO
OF69	86.71	FREE	NO
OF7	107.05	FREE	NO
OF70	86.88	FREE	NO
OF71	113.24	FREE	NO

OF72	85.7	FREE	NO
OF73	86.16	FREE	NO
OF74	86.29	FREE	NO
OF75	85.66	FREE	NO
OF76	85.25	FREE	NO
OF77	85.36	FREE	NO
OF78	85.52	FREE	NO
OF79	85.09	FREE	NO
OF8	107.47	FREE	NO
OF80	84.81	FREE	NO
OF81	85.11	FREE	NO
OF82	84.58	FREE	NO
OF83	84.44	FREE	NO
OF84	84.71	FREE	NO
OF85	84.78	FREE	NO
OF86	113.38	FREE	NO
OF87	84.01	FREE	NO
OF88	83.83	FREE	NO
OF89	84.19	FREE	NO
OF9	107.72	FREE	NO
OF90	83.5	FREE	NO
OF91	83.5	FREE	NO
OF92	83.79	FREE	NO
OF93	83.24	FREE	NO
OF94	82.76	FREE	NO
OF95	83.1	FREE	NO
OF96	83.35	FREE	NO
OF97	82.38	FREE	NO
OF98	82.18	FREE	NO
OF99	81.58	FREE	NO

[STORAGE]

;;Name	Elev.	MaxDepth	InitDepth	Shape	Curve Name/Params	N/A	
BioretentionPond	103	4	0	TABULAR	BioretentionPond	0	0
DENT1	81.01	3	0	TABULAR	DENT1	0	0
DENT1WQ	79.76	4.5	0	TABULAR	DENT2WQ	0	0
DENT2	85.14	3	0	TABULAR	DENT2	0	0
DENT2WQ	83.89	4.5	0	TABULAR	DENT2WQ	0	0
INFIL01	86.5	5.5	0	TABULAR	INFIL01	30	0

[CONDUITS]

;;Name	From Node	To Node	Length	Roughness	InOffset	OutOffset	In
C1	J11174	J11178	5.288	0.035	0	0	0
C10	J11164	J11173	6.275	0.035	0	0	0
C100	J11139	J11118	5.889	0.035	0	0	0
C1000	J10796	J10747	6.196	0.035	0	0	0
C10000	J7595	J7727	5.889	0.035	0	0	0
C10001	J7729	J7728	5.889	0.035	0	0	0
C10002	J7595	J7728	5.889	0.035	0	0	0
C10003	J7596	J7728	5.858	0.035	0	0	0
C10004	J7739	J7740	5.994	0.035	0	0	0
C10005	J7739	J7738	6.234	0.035	0	0	0

.....  
Too many conduit entities (32386 in total).

[ORIFICES]

;;Name	From Node	To Node	Type	Offset	Qcoeff	Gated	Cl
OR1	CB15	J8465	BOTTOM	2.8	0.8	NO	0
OR10	CB26	J7333	BOTTOM	3.76	0.8	NO	0
OR11	CB25	J7207	BOTTOM	3.363	0.8	NO	0
OR12	CB18	J6468	BOTTOM	3.7	0.8	NO	0



OR13	CB07	J3623	BOTTOM	3	0.8	NO	0
OR14	CB05	J3899	BOTTOM	2.6	0.8	NO	0
OR15	CB04	J2987	BOTTOM	3.2	0.8	NO	0
OR16	CB01	J3437	BOTTOM	3	0.8	NO	0
OR17	CB08	J3630	BOTTOM	3.4	0.8	NO	0
OR18	CB09	J4340	BOTTOM	3.868	0.8	NO	0
OR19	CB10	J4885	BOTTOM	3.6	0.8	NO	0
OR2	CB14	J7611	BOTTOM	3.418	0.8	NO	0
OR20	CB19	J3017	BOTTOM	3.3	0.8	NO	0
OR21	CB03	J4169	BOTTOM	3.4	0.8	NO	0
OR22	CB20	J4153	BOTTOM	3.5	0.8	NO	0
OR23	CB21	J3876	BOTTOM	3.137	0.8	NO	0
OR24	CB06	J4055	BOTTOM	2.675	0.8	NO	0
OR25	CB02	J3592	BOTTOM	2.9	0.8	NO	0
OR26	CB22	J5009	BOTTOM	2.9	0.8	NO	0
OR27	CB12	J4736	BOTTOM	4.035	0.8	NO	0
OR28	CB11	J4456	BOTTOM	3.928	0.8	NO	0
OR29	CB23	J4994	BOTTOM	4.1	0.8	NO	0
OR3	CB13	J7087	BOTTOM	3.753	0.8	NO	0
OR30	CB24	J5137	BOTTOM	3.544	0.8	NO	0
OR31	RunOff01	J6922	BOTTOM	0.1	1	NO	0
OR32	RunOff02	J9844	BOTTOM	0.21	1	NO	0
OR33	RunOff03	J9222	BOTTOM	0.1	1	NO	0
OR34	RunOff04	J8407	BOTTOM	0.33	1	NO	0
OR35	RunOff05	J7978	BOTTOM	0.35	1	NO	0
OR36	BioretentionPond	J9137	BOTTOM	3.5	0.8	NO	0
OR37	INFIL01	J7324	BOTTOM	7.033	1	NO	0
OR4	CB16	J8578	BOTTOM	3.901	0.8	NO	0
OR5	CB27	J7591	BOTTOM	5.731	0.8	NO	0
OR6	CB28	J7727	BOTTOM	4.6	0.8	NO	0
OR7	CB29	J7730	BOTTOM	4.237	0.8	NO	0
OR8	CB30	J7872	BOTTOM	4.839	0.8	NO	0
OR9	CB31	J8017	BOTTOM	3.598	0.8	NO	0

[XSECTIONS]

;;Link	Shape	Geom1	Geom2	Geom3	Geom4	Barrels	Culver
C1	RECT_OPEN	30	5.875	2	0	1	
C10	RECT_OPEN	30	5.705	2	0	1	
C100	RECT_OPEN	30	4.907	2	0	1	
C1000	RECT_OPEN	30	5.633	2	0	1	
C10000	RECT_OPEN	30	4.907	2	0	1	
C10001	RECT_OPEN	30	4.907	2	0	1	
C10002	RECT_OPEN	30	4.907	2	0	1	
C10003	RECT_OPEN	30	4.882	2	0	1	
C10004	RECT_OPEN	30	6.66	1	0	1	
C10005	RECT_OPEN	30	7.793	1	0	1	
.....							
Too many conduit entities (32386 in total).							
OR1	RECT_CLOSED	30	10	0	0		
OR10	RECT_CLOSED	30	10	0	0		
OR11	RECT_CLOSED	30	10	0	0		
OR12	RECT_CLOSED	30	10	0	0		
OR13	RECT_CLOSED	30	10	0	0		
OR14	RECT_CLOSED	30	10	0	0		
OR15	RECT_CLOSED	30	10	0	0		
OR16	RECT_CLOSED	30	10	0	0		
OR17	RECT_CLOSED	30	10	0	0		
OR18	RECT_CLOSED	30	10	0	0		
OR19	RECT_CLOSED	30	10	0	0		
OR2	RECT_CLOSED	30	10	0	0		
OR20	RECT_CLOSED	30	10	0	0		
OR21	RECT_CLOSED	30	10	0	0		
OR22	RECT_CLOSED	30	10	0	0		

OR23	RECT_CLOSED	30	10	0	0
OR24	RECT_CLOSED	30	10	0	0
OR25	RECT_CLOSED	30	10	0	0
OR26	RECT_CLOSED	30	10	0	0
OR27	RECT_CLOSED	30	10	0	0
OR28	RECT_CLOSED	30	10	0	0
OR29	RECT_CLOSED	30	10	0	0
OR3	RECT_CLOSED	30	10	0	0
OR30	RECT_CLOSED	30	10	0	0
OR31	RECT_CLOSED	30	10	0	0
OR32	RECT_CLOSED	30	10	0	0
OR33	RECT_CLOSED	30	10	0	0
OR34	RECT_CLOSED	30	10	0	0
OR35	RECT_CLOSED	30	10	0	0
OR36	RECT_CLOSED	30	10	0	0
OR37	RECT_CLOSED	30	10	0	0
OR4	RECT_CLOSED	30	10	0	0
OR5	RECT_CLOSED	30	10	0	0
OR6	RECT_CLOSED	30	10	0	0
OR7	RECT_CLOSED	30	10	0	0
OR8	RECT_CLOSED	30	10	0	0
OR9	RECT_CLOSED	30	10	0	0

[LOSSES]

;;Link	Kentry	Kexit	Kavg	Flap Gate	Seepage
;;-----	-----	-----	-----	-----	-----
C3616	0.5	0	0	NO	0
C3617	0.5	0	0	NO	0
C3618	0.5	0	0	NO	0
C3619	0.5	0	0	NO	0
C3620	0.5	0	0	NO	0
C3621	0.5	0	0	NO	0
C3622	0.5	0	0	NO	0
C3623	0.5	0	0	NO	0
C3624	0.5	0	0	NO	0
C3625	0.5	0	0	NO	0

.....

Too many conduit entities (32386 in total).

[CURVES]

;;Name	Type	X-Value	Y-Value
;;-----	-----	-----	-----
;Bioretention Pond, data from RIDEM Report here: "F:\P2021\0588\A10\background\client data\2012-0			
BioretentionPond	Storage	0	354
BioretentionPond		1.999	354
BioretentionPond		2	1073
BioretentionPond		3	2526
BioretentionPond		4	4112
;DENT1 Just the storage area of pipes on easter side of Detention Basin #1 from this plan here: "			
DENT1	Storage	0	1844.04
DENT1		3	1844.04
;DENT2 Just the storage area of pipes on easter side of Detention BASin #2 from this plan here: "			
DENT2	Storage	0	3528.36
DENT2		3	3528.36
;Storage Volume for the WQ portion of the Detention Basin #2 without pipes from this document :			
DENT2WQ	Storage	0	915
DENT2WQ		1.25	915
DENT2WQ		4.25	1445.167059
;Storage Curve for infiltration basin according to this plan: "F:\P2021\0588\A10\background\clie			
INFIL01	Storage	0	1330.56



INFIL01 5.5 1330.56

[TIMESERIES]

```
;;Name Date Time Value
;-----
;SCS_Type_III_5.03in design storm, total rainfall = 5.03 in, rain interval = 6 minutes, rain unit
10Year_5.03in 0:00 0.00503
10Year_5.03in 0:06 0.01006
10Year_5.03in 0:12 0.01509
10Year_5.03in 0:18 0.02012
10Year_5.03in 0:24 0.02515
10Year_5.03in 0:30 0.03018
10Year_5.03in 0:36 0.03521
10Year_5.03in 0:42 0.04024
10Year_5.03in 0:48 0.04527
10Year_5.03in 0:54 0.0503
10Year_5.03in 1:00 0.05533
10Year_5.03in 1:06 0.06036
10Year_5.03in 1:12 0.06539
10Year_5.03in 1:18 0.07042
10Year_5.03in 1:24 0.07545
10Year_5.03in 1:30 0.08048
10Year_5.03in 1:36 0.08551
10Year_5.03in 1:42 0.09054
10Year_5.03in 1:48 0.09557
10Year_5.03in 1:54 0.1006
10Year_5.03in 2:00 0.10568
10Year_5.03in 2:06 0.11081
10Year_5.03in 2:12 0.11604
10Year_5.03in 2:18 0.12132
10Year_5.03in 2:24 0.12671
10Year_5.03in 2:30 0.13214
10Year_5.03in 2:36 0.13767
10Year_5.03in 2:42 0.14325
10Year_5.03in 2:48 0.14894
10Year_5.03in 2:54 0.15467
10Year_5.03in 3:00 0.16051
10Year_5.03in 3:06 0.16639
10Year_5.03in 3:12 0.17238
10Year_5.03in 3:18 0.17841
10Year_5.03in 3:24 0.18455
10Year_5.03in 3:30 0.19074
10Year_5.03in 3:36 0.19703
10Year_5.03in 3:42 0.20336
10Year_5.03in 3:48 0.2098
10Year_5.03in 3:54 0.21629
10Year_5.03in 4:00 0.22288
10Year_5.03in 4:06 0.22952
10Year_5.03in 4:12 0.23626
10Year_5.03in 4:18 0.24305
10Year_5.03in 4:24 0.24994
10Year_5.03in 4:30 0.25688
10Year_5.03in 4:36 0.26392
10Year_5.03in 4:42 0.27102
10Year_5.03in 4:48 0.27821
10Year_5.03in 4:54 0.28545
10Year_5.03in 5:00 0.2928
10Year_5.03in 5:06 0.30019
10Year_5.03in 5:12 0.30769
10Year_5.03in 5:18 0.31523
10Year_5.03in 5:24 0.32288
10Year_5.03in 5:30 0.33057
10Year_5.03in 5:36 0.33837
10Year_5.03in 5:42 0.34621
```

10Year_5.03in	5:48	0.35416
10Year_5.03in	5:54	0.36216
10Year_5.03in	6:00	0.37036
10Year_5.03in	6:06	0.37876
10Year_5.03in	6:12	0.38746
10Year_5.03in	6:18	0.39636
10Year_5.03in	6:24	0.40557
10Year_5.03in	6:30	0.41498
10Year_5.03in	6:36	0.42468
10Year_5.03in	6:42	0.43459
10Year_5.03in	6:48	0.4448
10Year_5.03in	6:54	0.45522
10Year_5.03in	7:00	0.46593
10Year_5.03in	7:06	0.47684
10Year_5.03in	7:12	0.48806
10Year_5.03in	7:18	0.49948
10Year_5.03in	7:24	0.5112
10Year_5.03in	7:30	0.52312
10Year_5.03in	7:36	0.53534
10Year_5.03in	7:42	0.54777
10Year_5.03in	7:48	0.56049
10Year_5.03in	7:54	0.57342
10Year_5.03in	8:00	0.5868
10Year_5.03in	8:06	0.60073
10Year_5.03in	8:12	0.61527
10Year_5.03in	8:18	0.63036
10Year_5.03in	8:24	0.64605
10Year_5.03in	8:30	0.6623
10Year_5.03in	8:36	0.67915
10Year_5.03in	8:42	0.69655
10Year_5.03in	8:48	0.71456
10Year_5.03in	8:54	0.73312
10Year_5.03in	9:00	0.75229
10Year_5.03in	9:06	0.772
10Year_5.03in	9:12	0.79233
10Year_5.03in	9:18	0.8132
10Year_5.03in	9:24	0.83468
10Year_5.03in	9:30	0.85671
10Year_5.03in	9:36	0.87934
10Year_5.03in	9:42	0.90253
10Year_5.03in	9:48	0.92632
10Year_5.03in	9:54	0.95067
10Year_5.03in	10:00	0.97592
10Year_5.03in	10:06	1.00238
10Year_5.03in	10:12	1.03004
10Year_5.03in	10:18	1.05892
10Year_5.03in	10:24	1.089
10Year_5.03in	10:30	1.12028
10Year_5.03in	10:36	1.15278
10Year_5.03in	10:42	1.18648
10Year_5.03in	10:48	1.22138
10Year_5.03in	10:54	1.2575
10Year_5.03in	11:00	1.29653
10Year_5.03in	11:06	1.34019
10Year_5.03in	11:12	1.38848
10Year_5.03in	11:18	1.4414
10Year_5.03in	11:24	1.49894
10Year_5.03in	11:30	1.58093
10Year_5.03in	11:36	1.70718
10Year_5.03in	11:42	1.8777
10Year_5.03in	11:48	2.09248
10Year_5.03in	11:54	2.515
10Year_5.03in	12:00	2.93752
10Year_5.03in	12:06	3.1523

10Year_5.03in	12:12	3.32282
10Year_5.03in	12:18	3.44907
10Year_5.03in	12:24	3.53106
10Year_5.03in	12:30	3.5886
10Year_5.03in	12:36	3.64152
10Year_5.03in	12:42	3.68981
10Year_5.03in	12:48	3.73347
10Year_5.03in	12:54	3.7725
10Year_5.03in	13:00	3.80862
10Year_5.03in	13:06	3.84352
10Year_5.03in	13:12	3.87722
10Year_5.03in	13:18	3.90972
10Year_5.03in	13:24	3.941
10Year_5.03in	13:30	3.97108
10Year_5.03in	13:36	3.99996
10Year_5.03in	13:42	4.02762
10Year_5.03in	13:48	4.05408
10Year_5.03in	13:54	4.07933
10Year_5.03in	14:00	4.10368
10Year_5.03in	14:06	4.12747
10Year_5.03in	14:12	4.15066
10Year_5.03in	14:18	4.17329
10Year_5.03in	14:24	4.19532
10Year_5.03in	14:30	4.2168
10Year_5.03in	14:36	4.23767
10Year_5.03in	14:42	4.258
10Year_5.03in	14:48	4.27771
10Year_5.03in	14:54	4.29688
10Year_5.03in	15:00	4.31544
10Year_5.03in	15:06	4.33345
10Year_5.03in	15:12	4.35085
10Year_5.03in	15:18	4.3677
10Year_5.03in	15:24	4.38395
10Year_5.03in	15:30	4.39964
10Year_5.03in	15:36	4.41473
10Year_5.03in	15:42	4.42927
10Year_5.03in	15:48	4.4432
10Year_5.03in	15:54	4.45658
10Year_5.03in	16:00	4.46956
10Year_5.03in	16:06	4.48223
10Year_5.03in	16:12	4.49471
10Year_5.03in	16:18	4.50688
10Year_5.03in	16:24	4.51885
10Year_5.03in	16:30	4.53052
10Year_5.03in	16:36	4.54199
10Year_5.03in	16:42	4.55316
10Year_5.03in	16:48	4.56412
10Year_5.03in	16:54	4.57479
10Year_5.03in	17:00	4.58525
10Year_5.03in	17:06	4.59541
10Year_5.03in	17:12	4.60537
10Year_5.03in	17:18	4.61502
10Year_5.03in	17:24	4.62448
10Year_5.03in	17:30	4.63364
10Year_5.03in	17:36	4.64259
10Year_5.03in	17:42	4.65124
10Year_5.03in	17:48	4.65969
10Year_5.03in	17:54	4.66784
10Year_5.03in	18:00	4.67584
10Year_5.03in	18:06	4.68379
10Year_5.03in	18:12	4.69163
10Year_5.03in	18:18	4.69943
10Year_5.03in	18:24	4.70712
10Year_5.03in	18:30	4.71477



10Year_5.03in	18:36	4.72231
10Year_5.03in	18:42	4.72981
10Year_5.03in	18:48	4.7372
10Year_5.03in	18:54	4.74455
10Year_5.03in	19:00	4.75179
10Year_5.03in	19:06	4.75898
10Year_5.03in	19:12	4.76608
10Year_5.03in	19:18	4.77312
10Year_5.03in	19:24	4.78006
10Year_5.03in	19:30	4.78695
10Year_5.03in	19:36	4.79374
10Year_5.03in	19:42	4.80048
10Year_5.03in	19:48	4.80712
10Year_5.03in	19:54	4.81371
10Year_5.03in	20:00	4.8202
10Year_5.03in	20:06	4.82669
10Year_5.03in	20:12	4.83308
10Year_5.03in	20:18	4.83941
10Year_5.03in	20:24	4.8457
10Year_5.03in	20:30	4.85194
10Year_5.03in	20:36	4.85807
10Year_5.03in	20:42	4.86421
10Year_5.03in	20:48	4.87025
10Year_5.03in	20:54	4.87628
10Year_5.03in	21:00	4.88222
10Year_5.03in	21:06	4.8881
10Year_5.03in	21:12	4.89394
10Year_5.03in	21:18	4.89972
10Year_5.03in	21:24	4.90541
10Year_5.03in	21:30	4.91109
10Year_5.03in	21:36	4.91667
10Year_5.03in	21:42	4.92226
10Year_5.03in	21:48	4.92774
10Year_5.03in	21:54	4.93317
10Year_5.03in	22:00	4.93855
10Year_5.03in	22:06	4.94389
10Year_5.03in	22:12	4.94912
10Year_5.03in	22:18	4.95435
10Year_5.03in	22:24	4.95948
10Year_5.03in	22:30	4.96461
10Year_5.03in	22:36	4.96964
10Year_5.03in	22:42	4.97462
10Year_5.03in	22:48	4.97955
10Year_5.03in	22:54	4.98443
10Year_5.03in	23:00	4.98921
10Year_5.03in	23:06	4.99399
10Year_5.03in	23:12	4.99866
10Year_5.03in	23:18	5.00334
10Year_5.03in	23:24	5.00792
10Year_5.03in	23:30	5.01245
10Year_5.03in	23:36	5.01692
10Year_5.03in	23:42	5.02135
10Year_5.03in	23:48	5.02567
10Year_5.03in	23:54	5.03

;SCS\_Type\_III\_2.86in design storm, total rainfall = 2.86 in, rain interval = 6 minutes, rain unit

1Year_2.86	0:00	0.00286
1Year_2.86	0:06	0.00572
1Year_2.86	0:12	0.00858
1Year_2.86	0:18	0.01144
1Year_2.86	0:24	0.0143
1Year_2.86	0:30	0.01716
1Year_2.86	0:36	0.02002
1Year_2.86	0:42	0.02288

1Year_2.86	0:48	0.02574
1Year_2.86	0:54	0.0286
1Year_2.86	1:00	0.03146
1Year_2.86	1:06	0.03432
1Year_2.86	1:12	0.03718
1Year_2.86	1:18	0.04004
1Year_2.86	1:24	0.0429
1Year_2.86	1:30	0.04576
1Year_2.86	1:36	0.04862
1Year_2.86	1:42	0.05148
1Year_2.86	1:48	0.05434
1Year_2.86	1:54	0.0572
1Year_2.86	2:00	0.06009
1Year_2.86	2:06	0.06301
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1Year_2.86	2:30	0.07513
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1Year_2.86	2:54	0.08794
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1Year_2.86	16:00	2.54134
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1Year_2.86	16:12	2.55564
1Year_2.86	16:18	2.56256
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1Year_2.86	16:30	2.576
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1Year_2.86	16:54	2.60117
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1Year_2.86	17:48	2.64945
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1Year_2.86	18:00	2.65863
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1Year_2.86	18:48	2.69352
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1Year_2.86	19:18	2.71394
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1Year_2.86	23:00	2.83681
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1Year_2.86	23:42	2.85508
1Year_2.86	23:48	2.85754
1Year_2.86	23:54	2.86

;SCS\_Type\_III\_3.4in design storm, total rainfall = 3.4 in, rain interval = 6 minutes, rain units

2_Year_3.4in	0:00	0.034
2_Year_3.4in	0:06	0.034
2_Year_3.4in	0:12	0.034
2_Year_3.4in	0:18	0.034
2_Year_3.4in	0:24	0.034
2_Year_3.4in	0:30	0.034
2_Year_3.4in	0:36	0.034
2_Year_3.4in	0:42	0.034
2_Year_3.4in	0:48	0.034
2_Year_3.4in	0:54	0.034
2_Year_3.4in	1:00	0.034
2_Year_3.4in	1:06	0.034
2_Year_3.4in	1:12	0.034
2_Year_3.4in	1:18	0.034
2_Year_3.4in	1:24	0.034
2_Year_3.4in	1:30	0.034
2_Year_3.4in	1:36	0.034
2_Year_3.4in	1:42	0.034
2_Year_3.4in	1:48	0.034
2_Year_3.4in	1:54	0.034
2_Year_3.4in	2:00	0.03434
2_Year_3.4in	2:06	0.03468

2_Year_3.4in	2:12	0.03536
2_Year_3.4in	2:18	0.0357
2_Year_3.4in	2:24	0.03638
2_Year_3.4in	2:30	0.03672
2_Year_3.4in	2:36	0.0374
2_Year_3.4in	2:42	0.03774
2_Year_3.4in	2:48	0.03842
2_Year_3.4in	2:54	0.03876
2_Year_3.4in	3:00	0.03944
2_Year_3.4in	3:06	0.03978
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2_Year_3.4in	4:00	0.04454
2_Year_3.4in	4:06	0.04488
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2_Year_3.4in	4:18	0.0459
2_Year_3.4in	4:24	0.04658
2_Year_3.4in	4:30	0.04692
2_Year_3.4in	4:36	0.0476
2_Year_3.4in	4:42	0.04794
2_Year_3.4in	4:48	0.04862
2_Year_3.4in	4:54	0.04896
2_Year_3.4in	5:00	0.04964
2_Year_3.4in	5:06	0.04998
2_Year_3.4in	5:12	0.05066
2_Year_3.4in	5:18	0.051
2_Year_3.4in	5:24	0.05168
2_Year_3.4in	5:30	0.05202
2_Year_3.4in	5:36	0.0527
2_Year_3.4in	5:42	0.05304
2_Year_3.4in	5:48	0.05372
2_Year_3.4in	5:54	0.05406
2_Year_3.4in	6:00	0.05542
2_Year_3.4in	6:06	0.05678
2_Year_3.4in	6:12	0.05882
2_Year_3.4in	6:18	0.06018
2_Year_3.4in	6:24	0.06222
2_Year_3.4in	6:30	0.06358
2_Year_3.4in	6:36	0.06562
2_Year_3.4in	6:42	0.06698
2_Year_3.4in	6:48	0.06902
2_Year_3.4in	6:54	0.07038
2_Year_3.4in	7:00	0.07242
2_Year_3.4in	7:06	0.07378
2_Year_3.4in	7:12	0.07582
2_Year_3.4in	7:18	0.07718
2_Year_3.4in	7:24	0.07922
2_Year_3.4in	7:30	0.08058
2_Year_3.4in	7:36	0.08262
2_Year_3.4in	7:42	0.08398
2_Year_3.4in	7:48	0.08602
2_Year_3.4in	7:54	0.08738
2_Year_3.4in	8:00	0.09044
2_Year_3.4in	8:06	0.09418
2_Year_3.4in	8:12	0.09826
2_Year_3.4in	8:18	0.102
2_Year_3.4in	8:24	0.10608
2_Year_3.4in	8:30	0.10982



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2_Year_3.4in	10:24	0.20332
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2_Year_3.4in	13:48	0.17884
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2_Year_3.4in	23:06	0.0323
2_Year_3.4in	23:12	0.03162
2_Year_3.4in	23:18	0.03162
2_Year_3.4in	23:24	0.03094
2_Year_3.4in	23:30	0.0306
2_Year_3.4in	23:36	0.03026
2_Year_3.4in	23:42	0.02992
2_Year_3.4in	23:48	0.02924
2_Year_3.4in	23:54	0.02924

;SCS\_Type\_III\_lin design storm, total rainfall = 1 in, rain interval = 6 minutes, rain units = in

SCS_Type_III_lin	0:00	0.001
SCS_Type_III_lin	0:06	0.002
SCS_Type_III_lin	0:12	0.003
SCS_Type_III_lin	0:18	0.004
SCS_Type_III_lin	0:24	0.005
SCS_Type_III_lin	0:30	0.006
SCS_Type_III_lin	0:36	0.007
SCS_Type_III_lin	0:42	0.008
SCS_Type_III_lin	0:48	0.009
SCS_Type_III_lin	0:54	0.01
SCS_Type_III_lin	1:00	0.011
SCS_Type_III_lin	1:06	0.012
SCS_Type_III_lin	1:12	0.013
SCS_Type_III_lin	1:18	0.014
SCS_Type_III_lin	1:24	0.015
SCS_Type_III_lin	1:30	0.016
SCS_Type_III_lin	1:36	0.017
SCS_Type_III_lin	1:42	0.018
SCS_Type_III_lin	1:48	0.019
SCS_Type_III_lin	1:54	0.02
SCS_Type_III_lin	2:00	0.02101
SCS_Type_III_lin	2:06	0.02203
SCS_Type_III_lin	2:12	0.02307
SCS_Type_III_lin	2:18	0.02412
SCS_Type_III_lin	2:24	0.02519
SCS_Type_III_lin	2:30	0.02627
SCS_Type_III_lin	2:36	0.02737
SCS_Type_III_lin	2:42	0.02848
SCS_Type_III_lin	2:48	0.02961
SCS_Type_III_lin	2:54	0.03075
SCS_Type_III_lin	3:00	0.03191
SCS_Type_III_lin	3:06	0.03308
SCS_Type_III_lin	3:12	0.03427
SCS_Type_III_lin	3:18	0.03547
SCS_Type_III_lin	3:24	0.03669
SCS_Type_III_lin	3:30	0.03792



SCS_Type_III_lin	3:36	0.03917
SCS_Type_III_lin	3:42	0.04043
SCS_Type_III_lin	3:48	0.04171
SCS_Type_III_lin	3:54	0.043
SCS_Type_III_lin	4:00	0.04431
SCS_Type_III_lin	4:06	0.04563
SCS_Type_III_lin	4:12	0.04697
SCS_Type_III_lin	4:18	0.04832
SCS_Type_III_lin	4:24	0.04969
SCS_Type_III_lin	4:30	0.05107
SCS_Type_III_lin	4:36	0.05247
SCS_Type_III_lin	4:42	0.05388
SCS_Type_III_lin	4:48	0.05531
SCS_Type_III_lin	4:54	0.05675
SCS_Type_III_lin	5:00	0.05821
SCS_Type_III_lin	5:06	0.05968
SCS_Type_III_lin	5:12	0.06117
SCS_Type_III_lin	5:18	0.06267
SCS_Type_III_lin	5:24	0.06419
SCS_Type_III_lin	5:30	0.06572
SCS_Type_III_lin	5:36	0.06727
SCS_Type_III_lin	5:42	0.06883
SCS_Type_III_lin	5:48	0.07041
SCS_Type_III_lin	5:54	0.072
SCS_Type_III_lin	6:00	0.07363
SCS_Type_III_lin	6:06	0.0753
SCS_Type_III_lin	6:12	0.07703
SCS_Type_III_lin	6:18	0.0788
SCS_Type_III_lin	6:24	0.08063
SCS_Type_III_lin	6:30	0.0825
SCS_Type_III_lin	6:36	0.08443
SCS_Type_III_lin	6:42	0.0864
SCS_Type_III_lin	6:48	0.08843
SCS_Type_III_lin	6:54	0.0905
SCS_Type_III_lin	7:00	0.09263
SCS_Type_III_lin	7:06	0.0948
SCS_Type_III_lin	7:12	0.09703
SCS_Type_III_lin	7:18	0.0993
SCS_Type_III_lin	7:24	0.10163
SCS_Type_III_lin	7:30	0.104
SCS_Type_III_lin	7:36	0.10643
SCS_Type_III_lin	7:42	0.1089
SCS_Type_III_lin	7:48	0.11143
SCS_Type_III_lin	7:54	0.114
SCS_Type_III_lin	8:00	0.11666
SCS_Type_III_lin	8:06	0.11943
SCS_Type_III_lin	8:12	0.12232
SCS_Type_III_lin	8:18	0.12532
SCS_Type_III_lin	8:24	0.12844
SCS_Type_III_lin	8:30	0.13167
SCS_Type_III_lin	8:36	0.13502
SCS_Type_III_lin	8:42	0.13848
SCS_Type_III_lin	8:48	0.14206
SCS_Type_III_lin	8:54	0.14575
SCS_Type_III_lin	9:00	0.14956
SCS_Type_III_lin	9:06	0.15348
SCS_Type_III_lin	9:12	0.15752
SCS_Type_III_lin	9:18	0.16167
SCS_Type_III_lin	9:24	0.16594
SCS_Type_III_lin	9:30	0.17032
SCS_Type_III_lin	9:36	0.17482
SCS_Type_III_lin	9:42	0.17943
SCS_Type_III_lin	9:48	0.18416
SCS_Type_III_lin	9:54	0.189

SCS_Type_III_lin	10:00	0.19402
SCS_Type_III_lin	10:06	0.19928
SCS_Type_III_lin	10:12	0.20478
SCS_Type_III_lin	10:18	0.21052
SCS_Type_III_lin	10:24	0.2165
SCS_Type_III_lin	10:30	0.22272
SCS_Type_III_lin	10:36	0.22918
SCS_Type_III_lin	10:42	0.23588
SCS_Type_III_lin	10:48	0.24282
SCS_Type_III_lin	10:54	0.25
SCS_Type_III_lin	11:00	0.25776
SCS_Type_III_lin	11:06	0.26644
SCS_Type_III_lin	11:12	0.27604
SCS_Type_III_lin	11:18	0.28656
SCS_Type_III_lin	11:24	0.298
SCS_Type_III_lin	11:30	0.3143
SCS_Type_III_lin	11:36	0.3394
SCS_Type_III_lin	11:42	0.3733
SCS_Type_III_lin	11:48	0.416
SCS_Type_III_lin	11:54	0.5
SCS_Type_III_lin	12:00	0.584
SCS_Type_III_lin	12:06	0.6267
SCS_Type_III_lin	12:12	0.6606
SCS_Type_III_lin	12:18	0.6857
SCS_Type_III_lin	12:24	0.702
SCS_Type_III_lin	12:30	0.71344
SCS_Type_III_lin	12:36	0.72396
SCS_Type_III_lin	12:42	0.73356
SCS_Type_III_lin	12:48	0.74224
SCS_Type_III_lin	12:54	0.75
SCS_Type_III_lin	13:00	0.75718
SCS_Type_III_lin	13:06	0.76412
SCS_Type_III_lin	13:12	0.77082
SCS_Type_III_lin	13:18	0.77728
SCS_Type_III_lin	13:24	0.7835
SCS_Type_III_lin	13:30	0.78948
SCS_Type_III_lin	13:36	0.79522
SCS_Type_III_lin	13:42	0.80072
SCS_Type_III_lin	13:48	0.80598
SCS_Type_III_lin	13:54	0.811
SCS_Type_III_lin	14:00	0.81584
SCS_Type_III_lin	14:06	0.82057
SCS_Type_III_lin	14:12	0.82518
SCS_Type_III_lin	14:18	0.82968
SCS_Type_III_lin	14:24	0.83406
SCS_Type_III_lin	14:30	0.83833
SCS_Type_III_lin	14:36	0.84248
SCS_Type_III_lin	14:42	0.84652
SCS_Type_III_lin	14:48	0.85044
SCS_Type_III_lin	14:54	0.85425
SCS_Type_III_lin	15:00	0.85794
SCS_Type_III_lin	15:06	0.86152
SCS_Type_III_lin	15:12	0.86498
SCS_Type_III_lin	15:18	0.86833
SCS_Type_III_lin	15:24	0.87156
SCS_Type_III_lin	15:30	0.87468
SCS_Type_III_lin	15:36	0.87768
SCS_Type_III_lin	15:42	0.88057
SCS_Type_III_lin	15:48	0.88334
SCS_Type_III_lin	15:54	0.886
SCS_Type_III_lin	16:00	0.88858
SCS_Type_III_lin	16:06	0.8911
SCS_Type_III_lin	16:12	0.89358
SCS_Type_III_lin	16:18	0.896

SCS_Type_III_lin	16:24	0.89838
SCS_Type_III_lin	16:30	0.9007
SCS_Type_III_lin	16:36	0.90298
SCS_Type_III_lin	16:42	0.9052
SCS_Type_III_lin	16:48	0.90738
SCS_Type_III_lin	16:54	0.9095
SCS_Type_III_lin	17:00	0.91158
SCS_Type_III_lin	17:06	0.9136
SCS_Type_III_lin	17:12	0.91558
SCS_Type_III_lin	17:18	0.9175
SCS_Type_III_lin	17:24	0.91938
SCS_Type_III_lin	17:30	0.9212
SCS_Type_III_lin	17:36	0.92298
SCS_Type_III_lin	17:42	0.9247
SCS_Type_III_lin	17:48	0.92638
SCS_Type_III_lin	17:54	0.928
SCS_Type_III_lin	18:00	0.92959
SCS_Type_III_lin	18:06	0.93117
SCS_Type_III_lin	18:12	0.93273
SCS_Type_III_lin	18:18	0.93428
SCS_Type_III_lin	18:24	0.93581
SCS_Type_III_lin	18:30	0.93733
SCS_Type_III_lin	18:36	0.93883
SCS_Type_III_lin	18:42	0.94032
SCS_Type_III_lin	18:48	0.94179
SCS_Type_III_lin	18:54	0.94325
SCS_Type_III_lin	19:00	0.94469
SCS_Type_III_lin	19:06	0.94612
SCS_Type_III_lin	19:12	0.94753
SCS_Type_III_lin	19:18	0.94893
SCS_Type_III_lin	19:24	0.95031
SCS_Type_III_lin	19:30	0.95168
SCS_Type_III_lin	19:36	0.95303
SCS_Type_III_lin	19:42	0.95437
SCS_Type_III_lin	19:48	0.95569
SCS_Type_III_lin	19:54	0.957
SCS_Type_III_lin	20:00	0.95829
SCS_Type_III_lin	20:06	0.95958
SCS_Type_III_lin	20:12	0.96085
SCS_Type_III_lin	20:18	0.96211
SCS_Type_III_lin	20:24	0.96336
SCS_Type_III_lin	20:30	0.9646
SCS_Type_III_lin	20:36	0.96582
SCS_Type_III_lin	20:42	0.96704
SCS_Type_III_lin	20:48	0.96824
SCS_Type_III_lin	20:54	0.96944
SCS_Type_III_lin	21:00	0.97062
SCS_Type_III_lin	21:06	0.97179
SCS_Type_III_lin	21:12	0.97295
SCS_Type_III_lin	21:18	0.9741
SCS_Type_III_lin	21:24	0.97523
SCS_Type_III_lin	21:30	0.97636
SCS_Type_III_lin	21:36	0.97747
SCS_Type_III_lin	21:42	0.97858
SCS_Type_III_lin	21:48	0.97967
SCS_Type_III_lin	21:54	0.98075
SCS_Type_III_lin	22:00	0.98182
SCS_Type_III_lin	22:06	0.98288
SCS_Type_III_lin	22:12	0.98392
SCS_Type_III_lin	22:18	0.98496
SCS_Type_III_lin	22:24	0.98598
SCS_Type_III_lin	22:30	0.987
SCS_Type_III_lin	22:36	0.988
SCS_Type_III_lin	22:42	0.98899

SCS_Type_III_1in	22:48	0.98997
SCS_Type_III_1in	22:54	0.99094
SCS_Type_III_1in	23:00	0.99189
SCS_Type_III_1in	23:06	0.99284
SCS_Type_III_1in	23:12	0.99377
SCS_Type_III_1in	23:18	0.9947
SCS_Type_III_1in	23:24	0.99561
SCS_Type_III_1in	23:30	0.99651
SCS_Type_III_1in	23:36	0.9974
SCS_Type_III_1in	23:42	0.99828
SCS_Type_III_1in	23:48	0.99914
SCS_Type_III_1in	23:54	1

;SCS\_Type\_III\_2in design storm, total rainfall = 2 in, rain interval = 6 minutes, rain units = in

SCS_Type_III_2in	0:00	0.002
SCS_Type_III_2in	0:06	0.004
SCS_Type_III_2in	0:12	0.006
SCS_Type_III_2in	0:18	0.008
SCS_Type_III_2in	0:24	0.01
SCS_Type_III_2in	0:30	0.012
SCS_Type_III_2in	0:36	0.014
SCS_Type_III_2in	0:42	0.016
SCS_Type_III_2in	0:48	0.018
SCS_Type_III_2in	0:54	0.02
SCS_Type_III_2in	1:00	0.022
SCS_Type_III_2in	1:06	0.024
SCS_Type_III_2in	1:12	0.026
SCS_Type_III_2in	1:18	0.028
SCS_Type_III_2in	1:24	0.03
SCS_Type_III_2in	1:30	0.032
SCS_Type_III_2in	1:36	0.034
SCS_Type_III_2in	1:42	0.036
SCS_Type_III_2in	1:48	0.038
SCS_Type_III_2in	1:54	0.04
SCS_Type_III_2in	2:00	0.04202
SCS_Type_III_2in	2:06	0.04406
SCS_Type_III_2in	2:12	0.04614
SCS_Type_III_2in	2:18	0.04824
SCS_Type_III_2in	2:24	0.05038
SCS_Type_III_2in	2:30	0.05254
SCS_Type_III_2in	2:36	0.05474
SCS_Type_III_2in	2:42	0.05696
SCS_Type_III_2in	2:48	0.05922
SCS_Type_III_2in	2:54	0.0615
SCS_Type_III_2in	3:00	0.06382
SCS_Type_III_2in	3:06	0.06616
SCS_Type_III_2in	3:12	0.06854
SCS_Type_III_2in	3:18	0.07094
SCS_Type_III_2in	3:24	0.07338
SCS_Type_III_2in	3:30	0.07584
SCS_Type_III_2in	3:36	0.07834
SCS_Type_III_2in	3:42	0.08086
SCS_Type_III_2in	3:48	0.08342
SCS_Type_III_2in	3:54	0.086
SCS_Type_III_2in	4:00	0.08862
SCS_Type_III_2in	4:06	0.09126
SCS_Type_III_2in	4:12	0.09394
SCS_Type_III_2in	4:18	0.09664
SCS_Type_III_2in	4:24	0.09938
SCS_Type_III_2in	4:30	0.10214
SCS_Type_III_2in	4:36	0.10494
SCS_Type_III_2in	4:42	0.10776
SCS_Type_III_2in	4:48	0.11062
SCS_Type_III_2in	4:54	0.1135



SCS_Type_III_2in	5:00	0.11642
SCS_Type_III_2in	5:06	0.11936
SCS_Type_III_2in	5:12	0.12234
SCS_Type_III_2in	5:18	0.12534
SCS_Type_III_2in	5:24	0.12838
SCS_Type_III_2in	5:30	0.13144
SCS_Type_III_2in	5:36	0.13454
SCS_Type_III_2in	5:42	0.13766
SCS_Type_III_2in	5:48	0.14082
SCS_Type_III_2in	5:54	0.144
SCS_Type_III_2in	6:00	0.14726
SCS_Type_III_2in	6:06	0.1506
SCS_Type_III_2in	6:12	0.15406
SCS_Type_III_2in	6:18	0.1576
SCS_Type_III_2in	6:24	0.16126
SCS_Type_III_2in	6:30	0.165
SCS_Type_III_2in	6:36	0.16886
SCS_Type_III_2in	6:42	0.1728
SCS_Type_III_2in	6:48	0.17686
SCS_Type_III_2in	6:54	0.181
SCS_Type_III_2in	7:00	0.18526
SCS_Type_III_2in	7:06	0.1896
SCS_Type_III_2in	7:12	0.19406
SCS_Type_III_2in	7:18	0.1986
SCS_Type_III_2in	7:24	0.20326
SCS_Type_III_2in	7:30	0.208
SCS_Type_III_2in	7:36	0.21286
SCS_Type_III_2in	7:42	0.2178
SCS_Type_III_2in	7:48	0.22286
SCS_Type_III_2in	7:54	0.228
SCS_Type_III_2in	8:00	0.23332
SCS_Type_III_2in	8:06	0.23886
SCS_Type_III_2in	8:12	0.24464
SCS_Type_III_2in	8:18	0.25064
SCS_Type_III_2in	8:24	0.25688
SCS_Type_III_2in	8:30	0.26334
SCS_Type_III_2in	8:36	0.27004
SCS_Type_III_2in	8:42	0.27696
SCS_Type_III_2in	8:48	0.28412
SCS_Type_III_2in	8:54	0.2915
SCS_Type_III_2in	9:00	0.29912
SCS_Type_III_2in	9:06	0.30696
SCS_Type_III_2in	9:12	0.31504
SCS_Type_III_2in	9:18	0.32334
SCS_Type_III_2in	9:24	0.33188
SCS_Type_III_2in	9:30	0.34064
SCS_Type_III_2in	9:36	0.34964
SCS_Type_III_2in	9:42	0.35886
SCS_Type_III_2in	9:48	0.36832
SCS_Type_III_2in	9:54	0.378
SCS_Type_III_2in	10:00	0.38804
SCS_Type_III_2in	10:06	0.39856
SCS_Type_III_2in	10:12	0.40956
SCS_Type_III_2in	10:18	0.42104
SCS_Type_III_2in	10:24	0.433
SCS_Type_III_2in	10:30	0.44544
SCS_Type_III_2in	10:36	0.45836
SCS_Type_III_2in	10:42	0.47176
SCS_Type_III_2in	10:48	0.48564
SCS_Type_III_2in	10:54	0.5
SCS_Type_III_2in	11:00	0.51552
SCS_Type_III_2in	11:06	0.53288
SCS_Type_III_2in	11:12	0.55208
SCS_Type_III_2in	11:18	0.57312

SCS_Type_III_2in	11:24	0.596
SCS_Type_III_2in	11:30	0.6286
SCS_Type_III_2in	11:36	0.6788
SCS_Type_III_2in	11:42	0.7466
SCS_Type_III_2in	11:48	0.832
SCS_Type_III_2in	11:54	1
SCS_Type_III_2in	12:00	1.168
SCS_Type_III_2in	12:06	1.2534
SCS_Type_III_2in	12:12	1.3212
SCS_Type_III_2in	12:18	1.3714
SCS_Type_III_2in	12:24	1.404
SCS_Type_III_2in	12:30	1.42688
SCS_Type_III_2in	12:36	1.44792
SCS_Type_III_2in	12:42	1.46712
SCS_Type_III_2in	12:48	1.48448
SCS_Type_III_2in	12:54	1.5
SCS_Type_III_2in	13:00	1.51436
SCS_Type_III_2in	13:06	1.52824
SCS_Type_III_2in	13:12	1.54164
SCS_Type_III_2in	13:18	1.55456
SCS_Type_III_2in	13:24	1.567
SCS_Type_III_2in	13:30	1.57896
SCS_Type_III_2in	13:36	1.59044
SCS_Type_III_2in	13:42	1.60144
SCS_Type_III_2in	13:48	1.61196
SCS_Type_III_2in	13:54	1.622
SCS_Type_III_2in	14:00	1.63168
SCS_Type_III_2in	14:06	1.64114
SCS_Type_III_2in	14:12	1.65036
SCS_Type_III_2in	14:18	1.65936
SCS_Type_III_2in	14:24	1.66812
SCS_Type_III_2in	14:30	1.67666
SCS_Type_III_2in	14:36	1.68496
SCS_Type_III_2in	14:42	1.69304
SCS_Type_III_2in	14:48	1.70088
SCS_Type_III_2in	14:54	1.7085
SCS_Type_III_2in	15:00	1.71588
SCS_Type_III_2in	15:06	1.72304
SCS_Type_III_2in	15:12	1.72996
SCS_Type_III_2in	15:18	1.73666
SCS_Type_III_2in	15:24	1.74312
SCS_Type_III_2in	15:30	1.74936
SCS_Type_III_2in	15:36	1.75536
SCS_Type_III_2in	15:42	1.76114
SCS_Type_III_2in	15:48	1.76668
SCS_Type_III_2in	15:54	1.772
SCS_Type_III_2in	16:00	1.77716
SCS_Type_III_2in	16:06	1.7822
SCS_Type_III_2in	16:12	1.78716
SCS_Type_III_2in	16:18	1.792
SCS_Type_III_2in	16:24	1.79676
SCS_Type_III_2in	16:30	1.8014
SCS_Type_III_2in	16:36	1.80596
SCS_Type_III_2in	16:42	1.8104
SCS_Type_III_2in	16:48	1.81476
SCS_Type_III_2in	16:54	1.819
SCS_Type_III_2in	17:00	1.82316
SCS_Type_III_2in	17:06	1.8272
SCS_Type_III_2in	17:12	1.83116
SCS_Type_III_2in	17:18	1.835
SCS_Type_III_2in	17:24	1.83876
SCS_Type_III_2in	17:30	1.8424
SCS_Type_III_2in	17:36	1.84596
SCS_Type_III_2in	17:42	1.8494

SCS_Type_III_2in	17:48	1.85276
SCS_Type_III_2in	17:54	1.856
SCS_Type_III_2in	18:00	1.85918
SCS_Type_III_2in	18:06	1.86234
SCS_Type_III_2in	18:12	1.86546
SCS_Type_III_2in	18:18	1.86856
SCS_Type_III_2in	18:24	1.87162
SCS_Type_III_2in	18:30	1.87466
SCS_Type_III_2in	18:36	1.87766
SCS_Type_III_2in	18:42	1.88064
SCS_Type_III_2in	18:48	1.88358
SCS_Type_III_2in	18:54	1.8865
SCS_Type_III_2in	19:00	1.88938
SCS_Type_III_2in	19:06	1.89224
SCS_Type_III_2in	19:12	1.89506
SCS_Type_III_2in	19:18	1.89786
SCS_Type_III_2in	19:24	1.90062
SCS_Type_III_2in	19:30	1.90336
SCS_Type_III_2in	19:36	1.90606
SCS_Type_III_2in	19:42	1.90874
SCS_Type_III_2in	19:48	1.91138
SCS_Type_III_2in	19:54	1.914
SCS_Type_III_2in	20:00	1.91658
SCS_Type_III_2in	20:06	1.91916
SCS_Type_III_2in	20:12	1.9217
SCS_Type_III_2in	20:18	1.92422
SCS_Type_III_2in	20:24	1.92672
SCS_Type_III_2in	20:30	1.9292
SCS_Type_III_2in	20:36	1.93164
SCS_Type_III_2in	20:42	1.93408
SCS_Type_III_2in	20:48	1.93648
SCS_Type_III_2in	20:54	1.93888
SCS_Type_III_2in	21:00	1.94124
SCS_Type_III_2in	21:06	1.94358
SCS_Type_III_2in	21:12	1.9459
SCS_Type_III_2in	21:18	1.9482
SCS_Type_III_2in	21:24	1.95046
SCS_Type_III_2in	21:30	1.95272
SCS_Type_III_2in	21:36	1.95494
SCS_Type_III_2in	21:42	1.95716
SCS_Type_III_2in	21:48	1.95934
SCS_Type_III_2in	21:54	1.9615
SCS_Type_III_2in	22:00	1.96364
SCS_Type_III_2in	22:06	1.96576
SCS_Type_III_2in	22:12	1.96784
SCS_Type_III_2in	22:18	1.96992
SCS_Type_III_2in	22:24	1.97196
SCS_Type_III_2in	22:30	1.974
SCS_Type_III_2in	22:36	1.976
SCS_Type_III_2in	22:42	1.97798
SCS_Type_III_2in	22:48	1.97994
SCS_Type_III_2in	22:54	1.98188
SCS_Type_III_2in	23:00	1.98378
SCS_Type_III_2in	23:06	1.98568
SCS_Type_III_2in	23:12	1.98754
SCS_Type_III_2in	23:18	1.9894
SCS_Type_III_2in	23:24	1.99122
SCS_Type_III_2in	23:30	1.99302
SCS_Type_III_2in	23:36	1.9948
SCS_Type_III_2in	23:42	1.99656
SCS_Type_III_2in	23:48	1.99828
SCS_Type_III_2in	23:54	2

;SCS\_Type\_III\_3.4in design storm, total rainfall = 3.4 in, rain interval = 6 minutes, rain units

SCS_Type_III_3.4in	0:00	0.0034
SCS_Type_III_3.4in	0:06	0.0068
SCS_Type_III_3.4in	0:12	0.0102
SCS_Type_III_3.4in	0:18	0.0136
SCS_Type_III_3.4in	0:24	0.017
SCS_Type_III_3.4in	0:30	0.0204
SCS_Type_III_3.4in	0:36	0.0238
SCS_Type_III_3.4in	0:42	0.0272
SCS_Type_III_3.4in	0:48	0.0306
SCS_Type_III_3.4in	0:54	0.034
SCS_Type_III_3.4in	1:00	0.0374
SCS_Type_III_3.4in	1:06	0.0408
SCS_Type_III_3.4in	1:12	0.0442
SCS_Type_III_3.4in	1:18	0.0476
SCS_Type_III_3.4in	1:24	0.051
SCS_Type_III_3.4in	1:30	0.0544
SCS_Type_III_3.4in	1:36	0.0578
SCS_Type_III_3.4in	1:42	0.0612
SCS_Type_III_3.4in	1:48	0.0646
SCS_Type_III_3.4in	1:54	0.068
SCS_Type_III_3.4in	2:00	0.07143
SCS_Type_III_3.4in	2:06	0.0749
SCS_Type_III_3.4in	2:12	0.07844
SCS_Type_III_3.4in	2:18	0.08201
SCS_Type_III_3.4in	2:24	0.08565
SCS_Type_III_3.4in	2:30	0.08932
SCS_Type_III_3.4in	2:36	0.09306
SCS_Type_III_3.4in	2:42	0.09683
SCS_Type_III_3.4in	2:48	0.10067
SCS_Type_III_3.4in	2:54	0.10455
SCS_Type_III_3.4in	3:00	0.10849
SCS_Type_III_3.4in	3:06	0.11247
SCS_Type_III_3.4in	3:12	0.11652
SCS_Type_III_3.4in	3:18	0.1206
SCS_Type_III_3.4in	3:24	0.12475
SCS_Type_III_3.4in	3:30	0.12893
SCS_Type_III_3.4in	3:36	0.13318
SCS_Type_III_3.4in	3:42	0.13746
SCS_Type_III_3.4in	3:48	0.14181
SCS_Type_III_3.4in	3:54	0.1462
SCS_Type_III_3.4in	4:00	0.15065
SCS_Type_III_3.4in	4:06	0.15514
SCS_Type_III_3.4in	4:12	0.1597
SCS_Type_III_3.4in	4:18	0.16429
SCS_Type_III_3.4in	4:24	0.16895
SCS_Type_III_3.4in	4:30	0.17364
SCS_Type_III_3.4in	4:36	0.1784
SCS_Type_III_3.4in	4:42	0.18319
SCS_Type_III_3.4in	4:48	0.18805
SCS_Type_III_3.4in	4:54	0.19295
SCS_Type_III_3.4in	5:00	0.19791
SCS_Type_III_3.4in	5:06	0.20291
SCS_Type_III_3.4in	5:12	0.20798
SCS_Type_III_3.4in	5:18	0.21308
SCS_Type_III_3.4in	5:24	0.21825
SCS_Type_III_3.4in	5:30	0.22345
SCS_Type_III_3.4in	5:36	0.22872
SCS_Type_III_3.4in	5:42	0.23402
SCS_Type_III_3.4in	5:48	0.23939
SCS_Type_III_3.4in	5:54	0.2448
SCS_Type_III_3.4in	6:00	0.25034
SCS_Type_III_3.4in	6:06	0.25602
SCS_Type_III_3.4in	6:12	0.2619
SCS_Type_III_3.4in	6:18	0.26792



SCS_Type_III_3.4in	6:24	0.27414
SCS_Type_III_3.4in	6:30	0.2805
SCS_Type_III_3.4in	6:36	0.28706
SCS_Type_III_3.4in	6:42	0.29376
SCS_Type_III_3.4in	6:48	0.30066
SCS_Type_III_3.4in	6:54	0.3077
SCS_Type_III_3.4in	7:00	0.31494
SCS_Type_III_3.4in	7:06	0.32232
SCS_Type_III_3.4in	7:12	0.3299
SCS_Type_III_3.4in	7:18	0.33762
SCS_Type_III_3.4in	7:24	0.34554
SCS_Type_III_3.4in	7:30	0.3536
SCS_Type_III_3.4in	7:36	0.36186
SCS_Type_III_3.4in	7:42	0.37026
SCS_Type_III_3.4in	7:48	0.37886
SCS_Type_III_3.4in	7:54	0.3876
SCS_Type_III_3.4in	8:00	0.39664
SCS_Type_III_3.4in	8:06	0.40606
SCS_Type_III_3.4in	8:12	0.41589
SCS_Type_III_3.4in	8:18	0.42609
SCS_Type_III_3.4in	8:24	0.4367
SCS_Type_III_3.4in	8:30	0.44768
SCS_Type_III_3.4in	8:36	0.45907
SCS_Type_III_3.4in	8:42	0.47083
SCS_Type_III_3.4in	8:48	0.483
SCS_Type_III_3.4in	8:54	0.49555
SCS_Type_III_3.4in	9:00	0.5085
SCS_Type_III_3.4in	9:06	0.52183
SCS_Type_III_3.4in	9:12	0.53557
SCS_Type_III_3.4in	9:18	0.54968
SCS_Type_III_3.4in	9:24	0.5642
SCS_Type_III_3.4in	9:30	0.57909
SCS_Type_III_3.4in	9:36	0.59439
SCS_Type_III_3.4in	9:42	0.61006
SCS_Type_III_3.4in	9:48	0.62614
SCS_Type_III_3.4in	9:54	0.6426
SCS_Type_III_3.4in	10:00	0.65967
SCS_Type_III_3.4in	10:06	0.67755
SCS_Type_III_3.4in	10:12	0.69625
SCS_Type_III_3.4in	10:18	0.71577
SCS_Type_III_3.4in	10:24	0.7361
SCS_Type_III_3.4in	10:30	0.75725
SCS_Type_III_3.4in	10:36	0.77921
SCS_Type_III_3.4in	10:42	0.80199
SCS_Type_III_3.4in	10:48	0.82559
SCS_Type_III_3.4in	10:54	0.85
SCS_Type_III_3.4in	11:00	0.87638
SCS_Type_III_3.4in	11:06	0.9059
SCS_Type_III_3.4in	11:12	0.93854
SCS_Type_III_3.4in	11:18	0.9743
SCS_Type_III_3.4in	11:24	1.0132
SCS_Type_III_3.4in	11:30	1.06862
SCS_Type_III_3.4in	11:36	1.15396
SCS_Type_III_3.4in	11:42	1.26922
SCS_Type_III_3.4in	11:48	1.4144
SCS_Type_III_3.4in	11:54	1.7
SCS_Type_III_3.4in	12:00	1.9856
SCS_Type_III_3.4in	12:06	2.13078
SCS_Type_III_3.4in	12:12	2.24604
SCS_Type_III_3.4in	12:18	2.33138
SCS_Type_III_3.4in	12:24	2.3868
SCS_Type_III_3.4in	12:30	2.4257
SCS_Type_III_3.4in	12:36	2.46146
SCS_Type_III_3.4in	12:42	2.4941

SCS_Type_III_3.4in	12:48	2.52362
SCS_Type_III_3.4in	12:54	2.55
SCS_Type_III_3.4in	13:00	2.57441
SCS_Type_III_3.4in	13:06	2.59801
SCS_Type_III_3.4in	13:12	2.62079
SCS_Type_III_3.4in	13:18	2.64275
SCS_Type_III_3.4in	13:24	2.6639
SCS_Type_III_3.4in	13:30	2.68423
SCS_Type_III_3.4in	13:36	2.70375
SCS_Type_III_3.4in	13:42	2.72245
SCS_Type_III_3.4in	13:48	2.74033
SCS_Type_III_3.4in	13:54	2.7574
SCS_Type_III_3.4in	14:00	2.77386
SCS_Type_III_3.4in	14:06	2.78994
SCS_Type_III_3.4in	14:12	2.80561
SCS_Type_III_3.4in	14:18	2.82091
SCS_Type_III_3.4in	14:24	2.8358
SCS_Type_III_3.4in	14:30	2.85032
SCS_Type_III_3.4in	14:36	2.86443
SCS_Type_III_3.4in	14:42	2.87817
SCS_Type_III_3.4in	14:48	2.8915
SCS_Type_III_3.4in	14:54	2.90445
SCS_Type_III_3.4in	15:00	2.917
SCS_Type_III_3.4in	15:06	2.92917
SCS_Type_III_3.4in	15:12	2.94093
SCS_Type_III_3.4in	15:18	2.95232
SCS_Type_III_3.4in	15:24	2.9633
SCS_Type_III_3.4in	15:30	2.97391
SCS_Type_III_3.4in	15:36	2.98411
SCS_Type_III_3.4in	15:42	2.99394
SCS_Type_III_3.4in	15:48	3.00336
SCS_Type_III_3.4in	15:54	3.0124
SCS_Type_III_3.4in	16:00	3.02117
SCS_Type_III_3.4in	16:06	3.02974
SCS_Type_III_3.4in	16:12	3.03817
SCS_Type_III_3.4in	16:18	3.0464
SCS_Type_III_3.4in	16:24	3.05449
SCS_Type_III_3.4in	16:30	3.06238
SCS_Type_III_3.4in	16:36	3.07013
SCS_Type_III_3.4in	16:42	3.07768
SCS_Type_III_3.4in	16:48	3.08509
SCS_Type_III_3.4in	16:54	3.0923
SCS_Type_III_3.4in	17:00	3.09937
SCS_Type_III_3.4in	17:06	3.10624
SCS_Type_III_3.4in	17:12	3.11297
SCS_Type_III_3.4in	17:18	3.1195
SCS_Type_III_3.4in	17:24	3.12589
SCS_Type_III_3.4in	17:30	3.13208
SCS_Type_III_3.4in	17:36	3.13813
SCS_Type_III_3.4in	17:42	3.14398
SCS_Type_III_3.4in	17:48	3.14969
SCS_Type_III_3.4in	17:54	3.1552
SCS_Type_III_3.4in	18:00	3.16061
SCS_Type_III_3.4in	18:06	3.16598
SCS_Type_III_3.4in	18:12	3.17128
SCS_Type_III_3.4in	18:18	3.17655
SCS_Type_III_3.4in	18:24	3.18175
SCS_Type_III_3.4in	18:30	3.18692
SCS_Type_III_3.4in	18:36	3.19202
SCS_Type_III_3.4in	18:42	3.19709
SCS_Type_III_3.4in	18:48	3.20209
SCS_Type_III_3.4in	18:54	3.20705
SCS_Type_III_3.4in	19:00	3.21195
SCS_Type_III_3.4in	19:06	3.21681

SCS_Type_III_3.4in	19:12	3.2216
SCS_Type_III_3.4in	19:18	3.22636
SCS_Type_III_3.4in	19:24	3.23105
SCS_Type_III_3.4in	19:30	3.23571
SCS_Type_III_3.4in	19:36	3.2403
SCS_Type_III_3.4in	19:42	3.24486
SCS_Type_III_3.4in	19:48	3.24935
SCS_Type_III_3.4in	19:54	3.2538
SCS_Type_III_3.4in	20:00	3.25819
SCS_Type_III_3.4in	20:06	3.26257
SCS_Type_III_3.4in	20:12	3.26689
SCS_Type_III_3.4in	20:18	3.27117
SCS_Type_III_3.4in	20:24	3.27542
SCS_Type_III_3.4in	20:30	3.27964
SCS_Type_III_3.4in	20:36	3.28379
SCS_Type_III_3.4in	20:42	3.28794
SCS_Type_III_3.4in	20:48	3.29202
SCS_Type_III_3.4in	20:54	3.2961
SCS_Type_III_3.4in	21:00	3.30011
SCS_Type_III_3.4in	21:06	3.30409
SCS_Type_III_3.4in	21:12	3.30803
SCS_Type_III_3.4in	21:18	3.31194
SCS_Type_III_3.4in	21:24	3.31578
SCS_Type_III_3.4in	21:30	3.31962
SCS_Type_III_3.4in	21:36	3.3234
SCS_Type_III_3.4in	21:42	3.32717
SCS_Type_III_3.4in	21:48	3.33088
SCS_Type_III_3.4in	21:54	3.33455
SCS_Type_III_3.4in	22:00	3.33819
SCS_Type_III_3.4in	22:06	3.34179
SCS_Type_III_3.4in	22:12	3.34533
SCS_Type_III_3.4in	22:18	3.34886
SCS_Type_III_3.4in	22:24	3.35233
SCS_Type_III_3.4in	22:30	3.3558
SCS_Type_III_3.4in	22:36	3.3592
SCS_Type_III_3.4in	22:42	3.36257
SCS_Type_III_3.4in	22:48	3.3659
SCS_Type_III_3.4in	22:54	3.3692
SCS_Type_III_3.4in	23:00	3.37243
SCS_Type_III_3.4in	23:06	3.37566
SCS_Type_III_3.4in	23:12	3.37882
SCS_Type_III_3.4in	23:18	3.38198
SCS_Type_III_3.4in	23:24	3.38507
SCS_Type_III_3.4in	23:30	3.38813
SCS_Type_III_3.4in	23:36	3.39116
SCS_Type_III_3.4in	23:42	3.39415
SCS_Type_III_3.4in	23:48	3.39708
SCS_Type_III_3.4in	23:54	3.4

```
[REPORT]
;;Reporting Options
INPUT      NO
CONTROLS   NO
SUBCATCHMENTS ALL
NODES ALL
LINKS ALL
```

```
[TAGS]
Node      CB01      Connect2D
Node      CB02      Connect2D
Node      CB03      Connect2D
Node      CB04      Connect2D
Node      CB05      Connect2D
Node      CB06      Connect2D
```

Node CB07 Connect2D  
Node CB08 Connect2D  
Node CB09 Connect2D  
Node CB10 Connect2D

.....  
Too many tags (43714 in total).

[MAP]  
DIMENSIONS 380659.396282754 154407.039167156 382441.571121633 155363.667722307  
UNITS Feet

[COORDINATES]  
;;Node X-Coord Y-Coord  
;;-----  
CB01 381328.147 155109.63  
CB02 381394.467 155101.764  
CB03 381390.252 155068.783  
CB04 381400.76 155142.064  
CB05 381464.78 155088.21  
CB06 381576.482 155074.655  
CB07 381700.77 155098.859  
CB08 381768.421 155098.98  
CB09 381751.236 155060.738  
CB10 381748.452 155027.094

.....  
Too many junction entities (11240 in total).

[VERTICES]  
;;Link X-Coord Y-Coord  
;;-----

[POLYGONS]  
;;Subcatchment X-Coord Y-Coord  
;;-----  
SBioretentionPond 381983.211 154878.981  
SBioretentionPond 381999.074 154878.311  
SBioretentionPond 382008.686 154877.603  
SBioretentionPond 382022.795 154883.902  
SBioretentionPond 382029.22 154911.616  
SBioretentionPond 382035.458 154936.094  
SBioretentionPond 382038.517 154949.884  
SBioretentionPond 382043.366 154963.217  
SBioretentionPond 382087.609 154982.612  
SBioretentionPond 382142.156 154975.945  
SBioretentionPond 382191.855 154975.945  
SBioretentionPond 382303.372 154960.793  
SBioretentionPond 382318.524 154931.701  
SBioretentionPond 382334.888 154843.214  
SBioretentionPond 382271.856 154794.122  
SBioretentionPond 382169.43 154805.032  
SBioretentionPond 382134.884 154785.031  
SBioretentionPond 382009.941 154726.739  
SBioretentionPond 382006.545 154724.291  
SBioretentionPond 381999.224 154725.96  
SBioretentionPond 381990.637 154727.917  
SBioretentionPond 381978.528 154733.76  
SBioretentionPond 381978.074 154733.991  
SBioretentionPond 381971.776 154744.321  
SBioretentionPond 381968.752 154754.147  
SBioretentionPond 381965.981 154765.233  
SBioretentionPond 381963.839 154772.918  
SBioretentionPond 381960.816 154780.602  
SBioretentionPond 381963.461 154801.136  
SBioretentionPond 381962.957 154822.426



SBioretentionPond	381957.036	154845.605
SBioretentionPond	381951.745	154857.699
SBioretentionPond	381949.604	154865.131
SBioretentionPond	381950.486	154870.8
SBioretentionPond	381983.211	154878.981
SCB01	381326.59	155104.01
SCB01	381318.112	155119.884
SCB01	381321.919	155140.3
SCB01	381331.608	155176.981
SCB01	381343.027	155217.468
SCB01	381356.869	155249.996
SCB01	381378.324	155319.206
SCB01	381456.184	155316.783
SCB01	381512.392	155311.721
SCB01	381487.64	155295.102
SCB01	381414.092	155174.527
SCB01	381393.584	155143.764
SCB01	381397.827	155137.753
SCB01	381414.8	155127.852
SCB01	381417.876	155127.121
SCB01	381410.65	155122.389
SCB01	381388.191	155104.786
SCB01	381385.267	155104.653
SCB01	381326.59	155104.01
SCB02	381392.744	155098.109
SCB02	381388.191	155104.786
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SCB22	381681.024	155012.472
SCB22	381680.313	155012.562
SCB22	381666.699	155014.291
SCB23	381560.585	155026.571
SCB23	381558.715	155022.54
SCB23	381551.917	154997.763
SCB23	381553.957	154968.19
SCB23	381554.144	154963.311
SCB23	381529.163	154966.427
SCB23	381529.586	154979.549
SCB23	381508.951	154981.877
SCB23	381506.728	154975.951
SCB23	381498.58	154977.009
SCB23	381499.956	154998.808
SCB23	381485.67	155000.819
SCB23	381484.294	155004.734
SCB23	381482.486	155004.961
SCB23	381483.084	155007.96
SCB23	381494.047	155020.707
SCB23	381502.714	155035.749
SCB23	381535.092	155035.494
SCB23	381560.585	155026.571
SCB24	381627.379	155020.197
SCB24	381621.724	155013.626
SCB24	381619.185	155011.024
SCB24	381615.88	155007.636
SCB24	381611.369	154984.192
SCB24	381610.707	154980.753
SCB24	381611.11	154976.153
SCB24	381602.074	154977.326
SCB24	381595.195	154978.173
SCB24	381591.703	154968.014
SCB24	381583.872	154969.072
SCB24	381581.756	154959.866
SCB24	381554.144	154963.311
SCB24	381553.957	154968.19
SCB24	381551.917	154997.763
SCB24	381559.821	155026.571
SCB24	381599.846	155025.296
SCB24	381627.379	155020.197
SCB25	381589.671	154866.194
SCB25	381615.704	154860.427
SCB25	381611.83	154844.148
SCB25	381612.274	154844.04
SCB25	381586.052	154849.806
SCB25	381589.671	154866.194
SCB26	381615.704	154860.427
SCB26	381642.259	154854.544
SCB26	381640.924	154848.867
SCB26	381647.787	154847.509
SCB26	381645.397	154836.003
SCB26	381611.83	154844.148
SCB26	381615.704	154860.427
SCB27	381583.27	154829.627
SCB27	381581.038	154820.513
SCB27	381573.191	154788.469
SCB27	381485.214	154806.561
SCB27	381482.194	154807.182
SCB27	381480.741	154807.187
SCB27	381480.717	154807.192
SCB27	381478.134	154807.587
SCB27	381477.739	154807.705
SCB27	381476.825	154808.286



SCB27	381476.327	154808.603
SCB27	381476.214	154808.667
SCB27	381474.739	154810.091
SCB27	381474.672	154810.126
SCB27	381474.481	154810.192
SCB27	381472.27	154810.723
SCB27	381471.739	154810.598
SCB27	381471.276	154810.654
SCB27	381469.483	154810.936
SCB27	381468.739	154811.02
SCB27	381467.924	154811.119
SCB27	381468.101	154812.707
SCB27	381474.484	154870.143
SCB27	381521.07	154864.779
SCB27	381525.87	154861.391
SCB27	381543.092	154860.261
SCB27	381551.845	154906.283
SCB27	381576.438	154903.188
SCB27	381594.455	154900.754
SCB27	381589.494	154866.233
SCB27	381589.671	154866.194
SCB27	381586.052	154849.806
SCB27	381611.34	154844.246
SCB27	381606.153	154825.725
SCB27	381583.344	154829.929
SCB27	381583.27	154829.627
SCB28	381611.34	154844.246
SCB28	381612.274	154844.04
SCB28	381645.397	154836.003
SCB28	381641.894	154819.138
SCB28	381606.153	154825.725
SCB28	381611.34	154844.246
SCB29	381647.787	154847.509
SCB29	381681.118	154840.915
SCB29	381678.977	154828.185
SCB29	381680.43	154816.849
SCB29	381693.511	154812.779
SCB29	381686.372	154776.392
SCB29	381683.046	154777.026
SCB29	381674.907	154778.188
SCB29	381661.827	154784.293
SCB29	381656.013	154797.955
SCB29	381657.467	154816.267
SCB29	381641.894	154819.138
SCB29	381647.787	154847.509
SCB30	381681.118	154840.915
SCB30	381718.281	154833.563
SCB30	381713.038	154803.482
SCB30	381711.285	154793.418
SCB30	381744.901	154772.828
SCB30	381748.861	154770.402
SCB30	381781.17	154760.816
SCB30	381791.605	154757.72
SCB30	381803.532	154753.938
SCB30	381796.992	154752.609
SCB30	381783.558	154755.319
SCB30	381763.855	154759.294
SCB30	381748.336	154762.894
SCB30	381707.463	154772.375
SCB30	381706.593	154772.541
SCB30	381686.372	154776.392
SCB30	381693.511	154812.779
SCB30	381680.43	154816.849
SCB30	381678.977	154828.185

SCB30	381681.118	154840.915
SCB31	381718.281	154833.563
SCB31	381754.025	154826.492
SCB31	381754.535	154829.617
SCB31	381777.918	154816.569
SCB31	381799.996	154776.737
SCB31	381800.816	154775.258
SCB31	381801.725	154774.711
SCB31	381801.739	154774.661
SCB31	381801.909	154774.6
SCB31	381810.527	154769.413
SCB31	381816.242	154765.973
SCB31	381822.687	154762.094
SCB31	381832.277	154756.322
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SCB31	381852.258	154751.94
SCB31	381856.967	154751.064
SCB31	381830.121	154745.508
SCB31	381791.605	154757.72
SCB31	381748.861	154770.402
SCB31	381711.285	154793.418
SCB31	381718.281	154833.563
SDMH10	381754.535	154829.617
SDMH10	381754.025	154826.492
SDMH10	381640.924	154848.867
SDMH10	381642.259	154854.544
SDMH10	381589.494	154866.233
SDMH10	381594.478	154900.918
SDMH10	381598.996	154915.882
SDMH10	381599.134	154915.854
SDMH10	381755.412	154884.26
SDMH10	381754.848	154872.402
SDMH10	381782.235	154865.908
SDMH10	381782.726	154865.793
SDMH10	381779.885	154856.904
SDMH10	381760.872	154860.777
SDMH10	381754.535	154829.617
SDMH14	381195.213	154906.104
SDMH14	381236.102	154909.216
SDMH14	381351.738	154895.749
SDMH14	381354.999	154898.222
SDMH14	381356.564	154885.024
SDMH14	381419.117	154827.582
SDMH14	381419.138	154815.555
SDMH14	381352.616	154831.478
SDMH14	381293.509	154840.091
SDMH14	381210.344	154857.021
SDMH14	381174.999	154866.823
SDMH14	381169.058	154899.495
SDMH14	381167.91	154904.027
SDMH14	381175.607	154904.613
SDMH14	381180.518	154904.986
SDMH14	381190.989	154905.783
SDMH14	381195.213	154906.104
SDMH15	381168.136	155014.67
SDMH15	381168.267	155015.759
SDMH15	381182.954	155042.405
SDMH15	381187.745	155040.609
SDMH15	381196.777	155037.222
SDMH15	381226.936	155027.44
SDMH15	381227.102	155010.187
SDMH15	381231.601	155008.803
SDMH15	381248.903	154999.806
SDMH15	381281.777	154993.231

SDMH15	381286.84	154998.933
SDMH15	381284.008	154983.566
SDMH15	381295.506	154981.65
SDMH15	381295.122	154971.302
SDMH15	381308.536	154969.003
SDMH15	381307.386	154938.343
SDMH15	381349.674	154930.536
SDMH15	381351.988	154923.62
SDMH15	381355	154898.222
SDMH15	381351.738	154895.749
SDMH15	381236.102	154909.216
SDMH15	381175.607	154904.613
SDMH15	381165.584	154903.85
SDMH15	381154.853	154904.233
SDMH15	381168.136	155014.67
SDMH23	381611.752	154968.194
SDMH23	381611.726	154967.66
SDMH23	381717.579	154945.81
SDMH23	381715.32	154936.775
SDMH23	381773.764	154925.482
SDMH23	381766.988	154882.849
SDMH23	381783.929	154879.46
SDMH23	381782.235	154865.908
SDMH23	381754.848	154872.402
SDMH23	381755.412	154884.26
SDMH23	381599.134	154915.854
SDMH23	381599.322	154917.537
SDMH23	381604.508	154943.781
SDMH23	381598.476	154945.368
SDMH23	381602.074	154977.326
SDMH23	381611.573	154976.093
SDMH23	381611.752	154968.194
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SINFIL01	381312.546	154997.433
SINFIL01	381315.747	155018.511
SINFIL01	381317.33	155025.947
SINFIL01	381399.849	155015.316
SINFIL01	381484.294	155004.734
SINFIL01	381485.67	155000.819
SINFIL01	381499.956	154998.808
SINFIL01	381498.58	154977.009
SINFIL01	381506.728	154975.951
SINFIL01	381508.951	154981.877
SINFIL01	381529.586	154979.549
SINFIL01	381529.163	154966.427
SINFIL01	381581.756	154959.866
SINFIL01	381583.872	154969.072
SINFIL01	381591.703	154968.014
SINFIL01	381595.195	154978.173
SINFIL01	381602.074	154977.326
SINFIL01	381598.476	154945.368
SINFIL01	381604.508	154943.781
SINFIL01	381599.322	154917.537
SINFIL01	381599.134	154915.854
SINFIL01	381598.996	154915.882
SINFIL01	381594.478	154900.918
SINFIL01	381576.44	154903.188
SINFIL01	381551.845	154906.283
SINFIL01	381543.092	154860.261
SINFIL01	381525.87	154861.391
SINFIL01	381521.07	154864.779
SINFIL01	381474.484	154870.143
SINFIL01	381468.101	154812.707
SINFIL01	381419.678	154819.152

SINFIL01	381419.745	154822.095
SINFIL01	381420.061	154836.015
SINFIL01	381420.154	154836.435
SINFIL01	381420.445	154836.399
SINFIL01	381422.269	154846.069
SINFIL01	381428.11	154877.023
SINFIL01	381456.47	154873.957
SINFIL01	381457.62	154884.305
SINFIL01	381466.435	154885.071
SINFIL01	381474.483	154915.731
SINFIL01	381359.125	154930.678
SINFIL01	381355.892	154929.388
SINFIL01	381307.386	154938.343
SINFIL01	381308.536	154969.003
SINFIL01	381295.122	154971.302
SINFIL01	381295.506	154981.65
SINFIL01	381284.008	154983.566
SINFIL01	381286.84	154998.933
SINFIL01	381288.724	154999.426
SRunOff01	381101.46	155092.929
SRunOff01	381106.885	155077.129
SRunOff01	381181.214	155043.724
SRunOff01	381182.906	155043.164
SRunOff01	381179.187	155035.569
SRunOff01	381168.267	155015.759
SRunOff01	381168.136	155014.67
SRunOff01	381154.853	154904.233
SRunOff01	381165.584	154903.85
SRunOff01	381167.91	154904.027
SRunOff01	381169.058	154899.495
SRunOff01	381174.803	154867.9
SRunOff01	381134.072	154871.619
SRunOff01	381137.351	154788.172
SRunOff01	381098.385	154800.986
SRunOff01	381021.032	154817.109
SRunOff01	381031.535	154880.943
SRunOff01	381037.127	154978.8
SRunOff01	381046.073	155069.946
SRunOff01	381057.239	155077.539
SRunOff01	381096.116	155092.898
SRunOff01	381101.46	155092.929
SRunOff01	381101.46	155092.929
SRunOff01	381101.456	155092.939
SRunOff01	381101.677	155092.93
SRunOff01	381101.46	155092.929
SRunOff02	381727.26	154670.055
SRunOff02	381731.099	154739.169
SRunOff02	381731.34	154745.609
SRunOff02	381826.641	154725.867
SRunOff02	381986.514	154691.276
SRunOff02	382026.235	154686.986
SRunOff02	382050.443	154710.7
SRunOff02	382108.879	154703.97
SRunOff02	382304.882	154737.335
SRunOff02	382350.569	154738.119
SRunOff02	382357.438	154681.31
SRunOff02	382360.563	154608.261
SRunOff02	382265.638	154541.853
SRunOff02	382004.675	154593.262
SRunOff02	381814.612	154628.779
SRunOff02	381748.858	154643.658
SRunOff02	381727.26	154670.055
SRunOff03	381476.722	154728.61
SRunOff03	381485.361	154796.764

SRunOff03	381486.315	154806.335
SRunOff03	381573.191	154788.469
SRunOff03	381572.88	154787.199
SRunOff03	381591.192	154775.282
SRunOff03	381615.319	154770.05
SRunOff03	381672.291	154757.841
SRunOff03	381731.34	154745.609
SRunOff03	381731.099	154739.169
SRunOff03	381727.26	154670.055
SRunOff03	381689.823	154658.537
SRunOff03	381621.669	154658.537
SRunOff03	381580.873	154662.856
SRunOff03	381539.116	154668.616
SRunOff03	381495.44	154687.814
SRunOff03	381473.842	154700.773
SRunOff03	381476.722	154728.61
SRunOff04	381310.054	154837.68
SRunOff04	381352.616	154831.478
SRunOff04	381419.138	154815.555
SRunOff04	381419.134	154817.4
SRunOff04	381424.239	154818.545
SRunOff04	381468.101	154812.707
SRunOff04	381467.924	154811.119
SRunOff04	381468.739	154811.02
SRunOff04	381469.483	154810.936
SRunOff04	381471.276	154810.654
SRunOff04	381471.739	154810.598
SRunOff04	381472.27	154810.723
SRunOff04	381474.481	154810.192
SRunOff04	381474.672	154810.126
SRunOff04	381474.739	154810.091
SRunOff04	381476.214	154808.667
SRunOff04	381476.327	154808.603
SRunOff04	381476.825	154808.286
SRunOff04	381477.739	154807.705
SRunOff04	381478.134	154807.587
SRunOff04	381480.717	154807.192
SRunOff04	381480.741	154807.187
SRunOff04	381482.194	154807.182
SRunOff04	381485.214	154806.561
SRunOff04	381486.315	154806.335
SRunOff04	381485.361	154796.764
SRunOff04	381476.722	154728.61
SRunOff04	381473.842	154700.773
SRunOff04	381474.125	154700.604
SRunOff04	381316.896	154747.328
SRunOff04	381310.054	154837.68
SRunOff05	381134.033	154872.123
SRunOff05	381174.803	154867.9
SRunOff05	381174.999	154866.823
SRunOff05	381210.344	154857.021
SRunOff05	381293.509	154840.091
SRunOff05	381310.054	154837.68
SRunOff05	381316.896	154747.328
SRunOff05	381317.909	154747.027
SRunOff05	381194.027	154774.206
SRunOff05	381137.872	154787.645
SRunOff05	381134.033	154872.123

[SYMBOLS]

;;Gage X-Coord Y-Coord  
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### Order of Magnitude Level Opinion of Cost



