- With the structure operating at full capacity, the load on the foundation will be approximately 500 to 600 kips (neglecting buoyant effects).
- Net loading on the soil will approximately range from 100 to 200 kips.
- Site soil consists of over 20 feet of loose to medium dense non-engineered granular fill overlying a thin layer of organic silt
- Groundwater is anticipated to be approximately 3 to 6 feet below the ground surface.
- Depth required for over-excavation and replacement with structural fill would be in excess of 20 feet.

6.3.1 Seismic Activity

In accordance with the building code, a liquefaction analysis of the underlying site soils was conducted to determine the significance of liquefaction on the foundation system. Subsurface data collected at the site indicate portions of the underlying site soils may be susceptible to earthquake liquefaction. Considering the recommended foundation option will be a deep foundation extending below the potential zone for liquefaction, it is believed the structure will not be susceptible to impact due to liquefaction.

6.3.2 Excavation Support

Excavation support may be necessary in areas where the depth of the excavation is significant, or where the area disturbed by the excavation must be kept to a minimum. Recommendations addressing excavation support are provided below. Additional information on excavation support is available in Section 6.60 – Excavation Support, in the Geotechnical Report. Final recommendations regarding excavation support will be made during final design.

- All temporary excavated slopes in the underlying soils should be constructed not steeper than 1.5 horizontal to 1 vertical and in accordance with the requirements of the latest OSHA standards.
- Further excavation support will be required for steeper slopes.
- Significant dewatering will be required considering the proximity to the drainage channel. Sheet piling is recommended to facilitate the dewatering of the underlying soils during excavation for the foundation construction.



6.3.3 Dewatering

Significant dewatering and temporary diversion of the drainage channel is anticipated during construction of the structure. Primary recommendations to address dewatering are presented below.

- Based on the grain size analysis, it is recommended that dewatering of underlying soil should be handled by either drilled deep wells or wellpoints.
- Sheet piling and sandbags may be used in combination for temporary diversion of the drainage moat around the excavation area.
- 6.3.4 Vibration Induced Settlement, Preconstruction Survey, and Vibration Monitoring

The Easton Dam and Memorial Boulevard Bridge will be in close proximity to the site location for the UV disinfection system. Vibrations caused by installing Geopiers, driving piles, or installing sheeting in the vicinity of adjacent structures could result in densification of fill and associated settlement beneath those structures. Based on professional experience, the Geopier option has a lower potential for vibration-induced settlement than timber piles. Recommendations to address vibration concerns are presented below.

- Specifications should require a geotechnical engineer conduct a preconstruction survey of nearby structures prior to pile or pier installation.
- Vibrations should be monitored continuously by a seismograph during construction, specifically during all pile driving operations.
- Specifications should require establishment and monitoring of settlement control points adjacent to the proposed excavation and on nearby structures.

6.3.5 Subgrade Protection, Backfill, and Compaction

Significant amounts of over-excavation and subsequent backfilling below the structure is not likely to be required because a deep foundation system is recommended. Recommendations regarding subgrade protection and compaction are provided below.

 The contractor should be required to surface compact the subgrade around the Geopiers or piles and the entire footprint of the foundation prior to construction of the footing or pile cap.



- Compacted structural fill placed along sides of the structure to provide lateral resistance and frictional resistance against uplift.
- Compaction of fill in the structure footprint should be a minimum of 95 percent of the maximum dry density.
- Contractor should adjust lift thickness to meet required compaction.
- Soil should not be reused as structural fill intended for backfilling either below or around structures. Test soil samples do not contain adequate coarse gravel to meet recommended structural fill specifications.
- Soils not adequate for reuse as structural fill can be used in other areas not intended for support of structures or pavements.
- Soil analysis should be conducted on any soil considered for reuse.

6.4 Feasible Foundation Systems

Three feasible deep foundation systems were identified as feasible foundation types; rammed aggregate piers, timber piles and pressure injected footings. The following sections summarize each foundation system.

6.4.1 Rammed Aggregate Piers (Geopiers)

A Rammed Aggregate Pier (R.A.P.) foundation system is a proprietary system that may be feasible at this site. This is a shallow foundation system that transfers the foundation load to a deeper layer of soil if unsuitable soil is encountered at a shallow depth. The system does this by creating columns of compacted aggregate within the existing soil. Because there are no physical piles, or pile ends to engage, there is no need for a pile cap. The proposed structure can be designed as if it were being supported by competent soil.

6.4.1.1 Relative Costs for Rammed Aggregate Piers

Research into the cost for the installation of R.A.P. foundations has indicated that the current price varies from approximately \$30/lf to \$50/lf. It is anticipated that the size and type required for this project will likely cost approximately \$35/lf because the number of piles is not governed by pile capacity, but rather by location. Assuming an average R.A.P. length of 35 feet, a typical R.A.P. will cost approximately \$1225/each.



The use of R.A.P. foundations when appropriate often results in a 20% to 50% cost savings as compared to traditional deep foundation alternatives such as steel piles. In addition, the time required for the installation of R.A.P. foundations is often shorter than other traditional systems, reducing the overall time required for construction.

Using an assumed number of 75 piles for this project, the cost for the pile installation would be approximately \$91,875. Adding an additional 25% to account for mobilization/de-mobilization, and required soil testing would bring the total cost for this foundation system to approximately \$114,844.

6.4.2 Timber Piles

Timber piles derive their principal support from end-bearing in a competent layer of the soil strata. In this case, the glacial till encountered at depths of approximately 32 feet could be utilized for support of the anticipated compression loads. Timber piles, however, are not capable of providing significant resistance to tensile loads or uplift which will likely be a consideration during final design.

6.4.2.1 Relative Costs for Timber Piles

Timber piles are typically supplied in standard lengths to suit the needs of a specific project. For this project, it is anticipated that pile sections 35 to 40 feet in length can achieve the required capacity and are capable of meeting the requirements of the Geotechnical Engineer. This type of system is widely used in the area and supplies are readily available.

The current cost associated with supplying and installing timber piles of the anticipated length and capacity is approximately \$35/lf. Assuming an average pile length of 35 feet, a typical timber pile would cost approximately \$1225/each-in-place. Additional costs associated with the installation of timbers piles include mobilization / de-mobilization, and pre-drilling as needed to allow installation of the piles without damage. Using an assumed number of 75 piles for this project, and adding an additional 35% to account for mobilization / de-mobilization and pre-drilling would bring the total cost for this foundation system to approximately \$124,031.

Not included in the cost provided above are load tests. Pile load tests are not a requirement of the building code when the pile capacity is less than 40 tons. Due to the method of installation and available capacity equations based on blow counts, it is anticipated that a pile load test will not be necessary.



6.4.3 Pressure Injected Footings

Pressure Injected Footings (PIFs) are shallow piles that are installed by driving casing to a desired depth, injecting a ball of low slump concrete out the bottom of the casing by pounding, and forming a pile shaft as the outer casing is withdrawn.

6.4.3.1 Relative Costs for Pressure Injected Footings (P.I.F.s)

For this project, it is anticipated that P.I.F. sections 35 to 40 feet in length can achieve the required capacity and are capable of meeting the requirements provided by the Geotechnical Engineer. This type of system is widely used in the area and supplies are readily available.

The cost associated with the installation of P.I.F.s varies depending on the size, depth, and capacity requirements for a particular project. For this project, the cost per liner foot of P.I.F. is expected to be approximately \$45/lf, including mobilization / de-mobilization, and pre-drilling as needed to remove obstructions and allow installation. Assuming an average pile length of 35 feet, a P.I.F. would cost approximately \$1575/each. Using an assumed number of 75 piles the total cost for this foundation system to approximately \$118,125.

Not included in the cost provided above are load tests. Although load tests are not required by the building code for driven piles when the pile capacity is less than 40 tons, the only way to confirm the capacity of a P.I.F. is to conduct load tests. These tests will likely increase the cost associated with this system by an additional \$20,000.

6.5 Construction Monitoring

Monitoring is recommended during construction of the chosen foundation system. Recommendations include, but are not limited to:

- A preconstruction survey of structures within 200 feet of pile driving or pier/PIF installation.
- Dynamic analysis of pile or driving system to develop driving criteria.
- Monitoring of vibrations during pile driving or Geopier installation.
- Monitoring of settlement control points.



7.0 ENVIRONMENTAL ASSESSMENT AND PERMITTING

Appendix R provides a detailed environmental assessment for this project and identifies permits that will be necessary for implementation of project. The environmental assessment discusses both the direct and indirect environmental effects of the proposed system on the surrounding project area. Water quality improvement alternatives previously identified for this site are also discussed. A no build alternative forecast indicates the site will remain in an existing conditions state and untreated discharge from the Moat will continue to Easton's Bay. Under the no build alternative forecast, bacteria levels will likely persist as will closures at Easton Beach.

Implementation of the project is anticipated to result in a reduction in bacterial loading into Easton's Bay and will have a positive environmental effect on water quality and pollutant discharge concerns. Additionally, direct impacts during construction are anticipated to be minimal on the following:

- Daily activities
- Cultural resources
- Ecological resources
- Erosion and sedimentation
- Displacement
- Noise and Visual Impacts



8.0 CONCLUSIONS

The following paragraphs summarize the major recommendations for the UV disinfection system proposed for the Moat discharge that were generated by the preliminary design.

Design Criteria

- The system should be designed for a peak flow of 96 cubic feet per second. This peak flow is consistent with the Water Quality Volume that is regulated by the Rhode Island Department of Environmental Management (RIDEM) for the existing watershed. This storm equals or exceeds 93% of all storm events in the Newport area. While this flow does not include runoff from the Esplanade, Esplanade flow can be accommodated by the UV disinfection system, provided the total peak flow does not exceed 105 cfs, which is the maximum capacity of the single-channel system. Flow from the existing Moat and Esplanade watersheds are not necessarily additive, and Middletown should evaluate whether hydrologic controls allow the Esplanade to be discharged to Moat within the peak 105 cfs criteria.
- Disinfection criteria for effluent from the UV system should have a 30-day geometric mean of 104 cfu/100 ml. While the beach closure standard is 104 cfu/100 ml, the geometric mean accounts for 30-50% dilution available within 300 feet of the Moat outfall and is a more reasonable performance standard for manufacturers to achieve, as opposed to an absolute numerical limit.
- Since Easton's Bay provides only 30 to 50% dilution within 300 feet of the Moat outfall, no significant dilution (5:1, 10:1 or 50:1 ratios of Easton's Bay to Moat discharge) is considered for process design purposes.

Hydraulics

- The hydraulic analysis of the Moat and UV disinfection system showed an
 increase water surface elevations in the moat during design storms by
 about 24 to 30-inches, which will exacerbate flooding conditions along the
 Moat. The system cannot operate by gravity alone. A pump system is
 therefore required to convey water from the Moat to the UV disinfection
 system channel.
- With a pump system, the hydraulics of the Moat may also accept the discharge from the Esplanade without any significant additional flooding in the Moat.

• Moat and UV System hydraulics must be designed to account for future sea level rise. An average sea level rise of 0.65 inches per year was used in preliminary design, which is consistent with the worst-case projections that are being offered by the Rhode Island Coastal Resources Management Council (CRMC). At this rate after 25 years, high tide would be about 16.3 inches higher than current levels. A 16.3-inch sea level rise was incorporated into the preliminary design.

Siting

- The open grassed area just north of Memorial Blvd bridge is the most appropriate location for installation of the UV disinfection system. This location protects the UV disinfection system from shoaling and storm surges compared to alternative locations south of the bridge. The recommended single-channel configuration also minimizes the need to relocate existing utilities to the west of the pump station. Other UV channel configurations or locations to the north of this site will require either relocating existing sanitary force mains or creating two UV systems to treat the eastern and western portions of the moat.
- The geotechnical analysis found that a deep foundation system will be required for this site. Timber piles were identified as the most conventional system, and one that can be completed by several local contractors.

UV System Design

- A wide range of alternative UV disinfection systems were evaluated during the preliminary design. Our study found that only conventional UV systems have been developed to the point where they have the capacity to reliably manage the flows that are expected for this system. After screening potential systems, we issued a Request for Information (RFI) to five vendors to provide detailed information on their systems. One vendor who was the primary supplier of microwave systems in the United States only responded with a letter stating that the microwave technology has not yet been developed to the scale that it could be employed on this project.
- Based on evaluation of each UV vendor's response to our RFI, Trojan UV received the highest point total for the qualitative analysis. Wedeco, Aquionics and Calgon also submitted responses. The proposed Trojan UV system has the lowest estimated opinion of construction cost and 20-Year



Life Cycle Cost. Trojan UV was the only vendor to offer a life time guarantee to meet the 104 *Enterococci* colonies/100 ml RIDOH beach standard (30-day geometric mean). The UV disinfection system proposed by Trojan UV was the only single channel design, which results in the smallest foot print for construction and site disturbance. Given their small construction foot print, there is no apparent need for sewer line relocation under the selected Alternative location #2.

- All UV system vendors indicated their system designs are sensitive to flow. Significant increases in design flows will increase project footprint and costs. For example, the Trojan UV system with a single channel configuration as proposed could manage a peak flow with desired treatment results of up to 105 cfs. Peak design flow greater than 105 cfs will require adding another channel, in effect doubling UV channel footprint and increasing equipment costs.
- Pretreatment systems for large solids/particulate removal were reviewed and incorporated in preliminary design to minimize potential damage to the pump and downstream UV disinfection equipment. RFI's were provided to four vendors, and three responded. Lakeside Equipment Corporation appeared to have the best suited equipment for this project largely because its bar screen has no moving parts below the Moat water surface.
- In summary, this system will consist of a by-pass weir that will be automatically closed with rainfall and opened after the storm event passes. The pump and UV system will be activated and deactivated in conjunction with closing and opening the weir, respectively. Water diverted from the Moat by the by-pass weir will drain through the bar screen to a pump(s) that will convey the water up to the UV system. UV lamp intensity (i.e. UV dose) will be controlled using a UV transmittance sensor that will provide process feedback so that automated system adjustments provide the required UV dose.
- We have developed an opinion-of-construction cost of \$5.377 million and a 20-year life-cycle cost of \$21.624 million through preliminary design, which is based on current electricity costs to Newport and based on the premise of operating the UV lamps at 100% intensity for 48-hours during each of fifty four (54) rain events annually. Life cycle costs are conservative. These projections are higher than what was developed as part of the conceptual design. There are several reasons for this including:



- RIDOT and Middletown pump station outfalls must be relocated to discharge upstream of the Moat diversion weir.
- A pump station is required to prevent exacerbation of flooding along the Moat.
- An influent screen to remove large particulate matter and debris carried by the Moat.
- O National Grid will need to invest in some capital improvements in order to deliver the required power to this system. Preliminarily, National Grid has reported that \$150,000 to \$200,000 of capital improvements may be required to bring necessary service to the UV disinfection system. These costs may be shared with the City.
- o Deep foundations are required for this site.
- The area of influence within the Moat at the UV system intake and discharge will require reinforced construction using cable concrete along the Moat bottom.
- o A building to house the new electrical service is required due to its size.



TABLES

SUMMARY OF RESPONSES FOR UV DISINFECTION RFI UV DISINFECTION SYSTEM FOR STORMWATER TREATMENT TABLE 16

Newport, RI September 2008

	Aquionics Inc.	Trojan Technologies	Calgon Carbon Corp ¹	WEDECO UV Technologies, Inc
What is the name of the unit proposed for UV Disinfection?	Inline 18000+	Trojan UV4000Plus	C ⁵ 500 ^{FM}	Wedeco TAK55
What was the proposed capital cost? (Only 104 <i>Enterwaci</i> /100 ml design	At 96 cfs: \$1,992,000	At 96 cfs: \$1,650,000:	At 96 cfs: \$3,054,100 (updated price)	At 96 cfs: \$3,200,000
basis shown)	At 116 cfs: \$2,316,000	At 116 cfs: \$3,084000	•	At 116 cfs: \$3,700,000
Was a list provided of UV installations of the same magnitude for similar systems?	Yes.	Yes.	No.	Yes.
What was the relative design flow?	183 total installations <10 MGD: 164 10 to 50 MGD: 17 >50 MGD: 2	286 total installations <10 MGD: 105 10 to 50 MGD: 153 >50 MGD: 28	35 total installations <10 MGD: 33 10 to 50 MGD: 2 >50 MGD: 0	251 total installations <10 MGD: 186 10 to 50 MGD: 54 >50 MGD: 11
What kind of water stream was treated (stream, wastewater, stormwater, etc)?	Municipal Wastewater and Municipal Drinking Water	Combined Sewer Overflow & Sanitary Sewer Overflow	Water and Wastewater	Sanitary wastewater

	Aquionics Inc.	Trojan Technologies	Calgon Carbon Corp ¹	WEDECO UV Technologies, Inc
What operating data such as bacterial counts per 100 mL, Total Suspended Solids, daily flow rate, transmissivity, etc were provided?	Peak flow, Toral Suspended Solids, Ultraviolet transmissibility, 3-log reduction counts	Peak design flow, TSS, Ultraviolet transmissibility, Disinfection limit, Design dose, Fouling factor	Peak Design Flow	Peak design flow, TSS, Ultraviolet transmissibility, Disinfection limit, Design dose
What was down time as a function of total operating time?	Not discussed in vendor response.	Information not provided to Manufacturer by clients	Not discussed in vendor response.	Not discussed in vendor response.
What is the provided electrical cost?	Not discussed in vendor response.	At 96 cfs: \$207,336 At 116 cfs: \$189,136 (based on 40, 48-hour long events and \$0.10/kWH)	Not discussed in vendor updated response.	Not discussed in vendor response.
What is the electrical load at 100% lamp power? (Only 104 Enterwari/100 ml design basis shown)	At 96 cfs: 1361 kW (Total system)	At 96 cfs: 1,408 kW (Total System)	At 96 cfs: 768 kW (Lamps and Ballast only)	At 96 cfs: 700 kW (Lamps & Ballasts only)
What was projected Operation and Maintenance Cost?	Not discussed in vendor response.	Not discussed in vendor response.	Not discussed in vendors updated response.	Not discussed in vendor response.
What Environmental Standards needed to be met for the installation?	Not discussed in vendor response.	Not discussed in vendor response.	Not discussed in vendor response.	Not discussed in vendor response.

	Aquionics Inc.	Trojan Technologies	Calgon Carbon Corp¹	WEDECO UV Technologies, Inc
How should removal of grit	Screening	Screening and grit	Not discussed in	Protective baffle plate
and solids prior to your UV	recommended both	removal recommended	vendor response.	ahead of the first lamp
disinfection system be	upstream &	both upstream &	•	bank included which is
handled? It is important to	downstream of the unit	downstream of the unit		a 1" punch plate.
note that the stormwater	# # # # # # # # # # # # # # # # # # #			
discharge carries debris and	AQUIONICS does not	TROJAN does not		WEDECO
solids like tree limbs, shoes,	provide screening/grit	provide screening/grit		recommends screening
and trash. The discharge is	separation	separation		& grit removal to 30µ
tidally influenced so				with a maximum TSS
sand/grit and seaweed may be carried back into the UV				of 30 mg/L
system channel				WEDECO door and
System Chambel.				WEDECO does not
				provide screening/grit
				separation
What is the headloss across	12" per unit	At 96 cfs: 16.78 inches	At 96 cfs: 28.21 inches	At 96 cfs: 18.9 inches
the UV disinfection system				
in inches of water for the	(12 units recommended	(1 channels required)	(5 channels required)	At 116 cfs: 23.4 inches
specified flow rates? (Only	operated in parallel)			
104 Enterocai/100 ml				(4 to 5 channels
design basis shown)				required)
Provide detail information	PLC controlled (data	PLC controlled (data	PLC controlled (data	PLC controlled (data
on the type of monitoring	provided) and included	provided) and included	gleamed from	provided) and included
and control systems used to	in system cost.	in system cost.	specification and	in system cost.
operate the system.			included in system cost.	
What level of experience do	Basic knowledge of	Basic knowledge of	Basic knowledge of	3-5 days training by
operators typically need to	wastewater equipment;	wastewater equipment;	wastewater equipment;	factory rep, basic
use equipment?	equipment-specific	equipment-specific	equipment-specific	computer skills.
	ruming provident	Transport of the saction	Transaction (
Can the system be nooked	1 es, supervisory	res, supervisory	res, supervisory	x es, supervisory
up to a communication line	control and data	control and data	control and data	control and data
for remote operation/ alarm	acquisition (SCADA) interface provided.	acquisition (SCADA) interface provided	acquisition (SCADA)	acquisition (SCADA)
			Transport of the state of the s	ALARCA ARCO PLANT ALLONIA

	Aquionics Inc.	Trojan Technologies	Calgon Carbon Corp ¹	WEDECO UV
				recmologies, inc
How much attention does	Koutine inspection &	Koutine inspection &	Routine inspection &	System may be
the system require? Will the system need to be	maintenance.	maintenance.	maintenance.	operated unattended.
continuously attended	System may be	System may be	System may be	System has remote
during operation?	operated unattended	operated unattended.	operated unattended.	alarm capability
		Weekly inspection		required).
		recommended		
		regardless of if		
		equipment is used or		
How readily available are	All components	On-site service	Parts are readably	Replacement parts
system replacement	available from upstate	available locally from	available out of their	shipped 24-48 hours
components? Are they off	NY office.	MAHER in	Pittsburg PA office.	after receipt of payment
the shelf parts that can be		Massachusetts, who is a		or purchase order,
ordered for rush delivery in		factory-authorized field		location not indicated
24 hours? Please provide a		service provider.		in vendor response.
ust of suppliers capable of				
supplying all system				
Components.	Vac a concil and district	Voc o romall and strike	N.5.	, N
chemical cleaning or	les, a sman quantity or lamo cleaning solution.	lamo cleaning solution.		ONT
chemicals for normal			(System utilizes Teflon	(System utilizes Teflon
operations?	(The composition and	(The composition and	wiper blades to clean	wiper blades to clean
	exact volume of the	exact volume of the	lamp sleeves and does	lamp sleeves and does
	solution is not stated).	solution is not stated,	not require chemicals)	not require chemicals).
		but is rood grade based).		
Is an automatic means of	Yes.	Yes.	Yes.	Yes.
cleaning the lamps				

	Aquionics Inc.	Trojan Technologies	Calgon Carbon Corp¹	WEDECO UV Technologies, Inc
How often are UV lamps changed out under normal	4,000 to 8,000 hours	~ 5,000 hours	~12,000 hours	~ 12,000 hours
operating conditions (e.g. hours of operation)?	Lamp replacement cost is \$500 each	Lamp replacement cost is \$300 each	Lamp replacement cost is \$250 each	Lamp replacement cost is \$199 each
What is the normal cycling	Up to 6 on/off cycles	Up to 4 on/off cycles	Not discussed in	4 on/off cycles per 24-
on/off time for UV lamps and will prolonged	per day	per day	vendor response.	hr. period.
shutdown followed by	Manufacturer	Prolonged shutdown		
startup adversely impact the	recommends a	does not affect lamp		
samps:	numinum of monthly startup of lamps.	lire.		
How is the system mounted	No channel mounting	Cast-in-place concrete	Mounts in a cast-in-	Mounts in a cast-in-
in the treatment channel.	provided (UV lamps are	around a tactory- fabricated steel insert	place concrete channel	place concrete channel
	mounted in provided			
	pipe)			
Can the system be	A "dogleg" loop at the	An adjustable weir plate	An adjustable weir gate	An adjustable weir plate
Contigured for varying water	outlet of the system is	15 provided.	ıs provided.	ıs provided.
be fully submerged at all	recommended.	Lamps must be	Lamps must be	Lamps must be
times?	Lamps must be	submerged during	submerged during	submerged during
	submerged during	operation.	operation.	operation.
Will the ballast of the lamps	System does not utilize	Yes, ballasts are not	Yes, ballasts are not	Yes, ballasts are not
need to be replaced if	ballast, but different	designed for	designed for	designed for
submerged? If so, provide a description of required	technology, which is	submergence.	submergence.	submergence.
equipment replacement and	submergence.	Ballast replacement	Ballast replacement	Ballast replacement
budgetary costs.		cost is \$970 each	cost is \$400 each	cost is \$350 each
	Ballast replacement cost is Not available			

Notes:

Responses for vendor based on a follow up phone conversation and revised quote. Original quote was based on killing feat whi form bacteria, not Enterwayi

Page of

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UV DISINECTION SYSTEM QUALITATIVE ANALYSIS

UV DISINFECTION SYSTEM FOR STORMWATER TREATMENT

96 cfs (62 MGD) Design Flow

Newport, RI

September 2008 TABLE 17

	Delice Vieles Darle and to The A.	Aquionics	orics	Trojan UV.	1 U.V.	osapaM	deco	Č	Calgon
equested	round value assigned to Each Requested Item	Response to Question	Resulting Numeric Value	Response to Question	Resulting Numeric Value	Response to Ouestion	Resulting Numeric Value	Response to Ouestion	Resulting Numeric Value
tection Experience at	10 pts for a Yes	Yes	10 pts		10 pts	Yes	10 pts	No.	0 pts
of ongoing UV	15 pts for a Yes	Yes	15 pts	Yes	15 pts	No	0 pts	N _o	sud ()
g data for its list of	10 pts for a Yes	Yes	10 pts	Yes	10 pts	Yes	10 pts	1 es	10 pts
engineering?	5 pts for a Yes	Yes	sid č	Yes	5 pts	Yes	5 pts	°N	0 pts
s guarantee meeting standard?	15 pts for a Nes	Yes	15 pts	Yes	15 pts	No.	0 pts	Š	0 pts
namee?	10 pts for a Yes	Yes	10 pts	ŞəŢ	10 pts	Yes	10 pts	No	0 pts
were provided?		185	2 pts	586	10 pts	251	5 pts	35	S pts
han or equal to 10	Number of Points equals the number of installations in a given	164	18 pts	105	7 pts	186	15 pts	33	19 pts
een 10 and 50 MGD	size range, divided by the total number of installations, times 20	ţ.	2 pts	153	11 pts	40	4 pts	2	1 pts
ations greater than or ded?		2	0 pts	28	2 pts	17	1 pts	0	0 pts
lloss (inches) for the	-0,2 pt for each inch of headloss	12.0 inches	-2 pts	16.78 inches	-3 pts	18.90 inches	-4 pts	28.21 inches	s1d 9-
to operate the lamps	-0.1 pt for each 100 kw	1,361 kw	⊸1 pts	1,075 kw	-1 pts	700 kw	-1 pts	786 kw	-1 pts
e the treatment	-10 pts for a Yes	Yes	-10 pts	No	0 pts	Yes	-10 pts	Yes	-10 pts
ıd/or drawings	5 prs for a Yes	Yes	sid č	Yes	5 pts	Yes	std ç	Yes	5 pts
for normal system	-5 pts for a Yes	Σes	-5 pts	Yes	sid č-	No	0 pts	No	0 pts
for the operating life 3	10 points for the highest, 5 points for the next highest, and 2 points	8,000 hours	5 pts	5,000 hours	2 prs	12,000 hours	10 pts	12,000 hours	10 pts
re needed for 4 col/100mF		(12 units - 35"L x 42" W each)		(1 Channel - 96"W x 39L x		(4 Channels)		(5 Channels - 44'Wx38Lx78"D)	
d for treatment at 62		216 Lamps		336 Lamps		1944 Lamps		1560 Lamps	
costĉ	-1 pt for each \$1,000,000	\$ 24,005,762.29	-24 pts	\$ 21,624,080.28	-22 pts	\$ 22,245,207.13	-22 prs	\$ 22,802,615.63	-23 pts
			54.2 prs		92.6 pts		60.5 prs		33.6 pts

rport Tables Talles 17 - all LRM, all RFI Evabarion Calculation UV Disinfection, N930716.33